

**NCHRP Project 9-54**  
**Long-Term Aging of Asphalt**  
**Mixtures for Performance Testing**  
**and Prediction**

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Bozeman, MT  
Sep. 20, 2017

# Outline

- ❑ NCHRP 09-54 objectives
- ❑ Proposed Long-Term Aging Method
- ❑ Proposed Aging Duration Maps Based on Climatic Data
- ❑ Pavement Aging Model for M-E Analysis and Design
- ❑ Summary

# NCHRP 09-54 Objectives

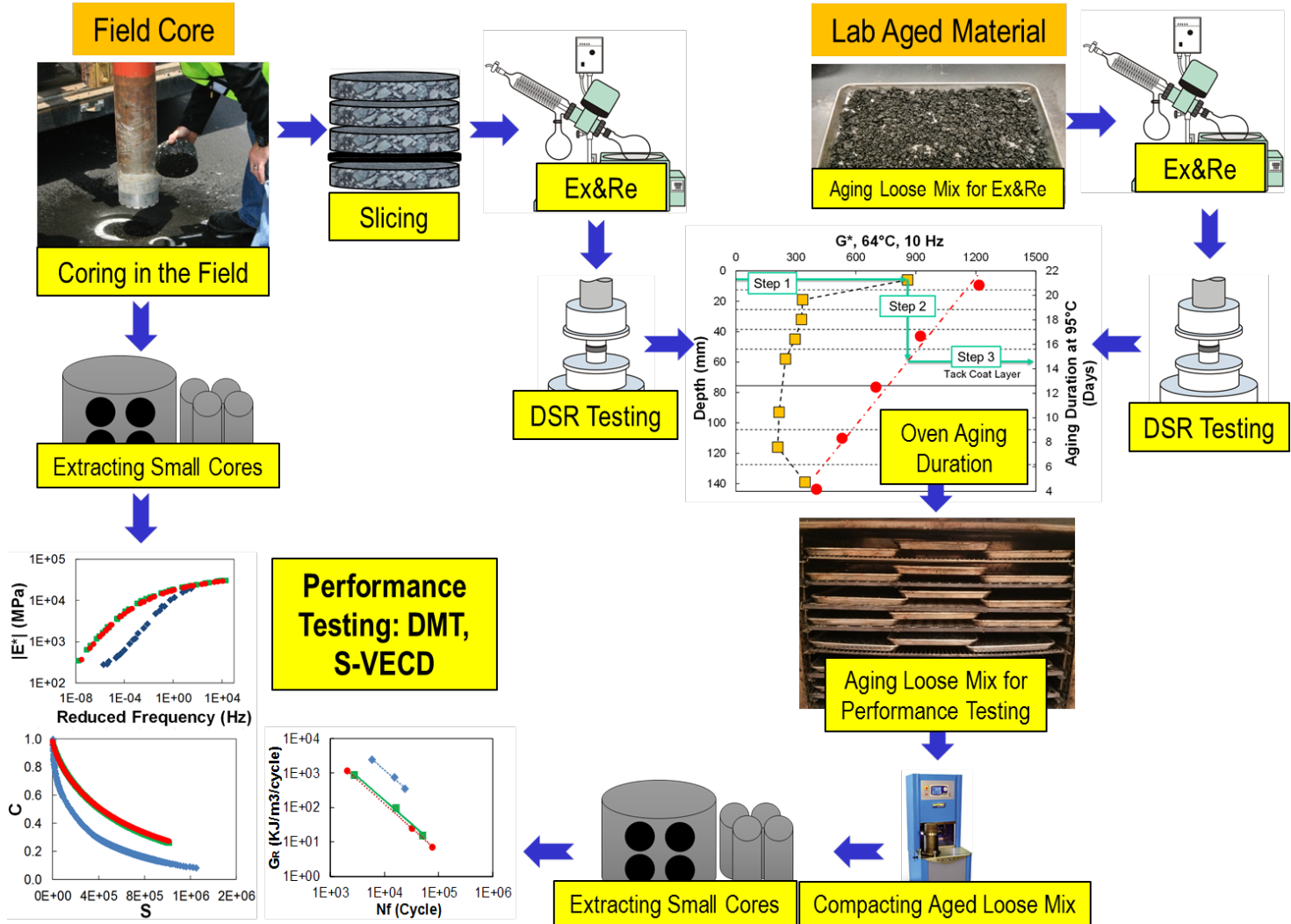
- ❑ Develop a calibrated and validated procedure to simulate long-term aging of asphalt mixtures for performance testing and prediction.
- ❑ Develop a pavement aging model as a function of climate, age, and pavement depth.

# Proposed Long-Term Aging Method

- ❑ Oven aging
- ❑ Loose mix aging
- ❑ 95°C



# Experimental Steps



# NCHRP 9-54 Kinetics Model

$$\log G^* = \log G_0^* + M \left(1 - \frac{k_c}{k_f}\right) \left(1 - \exp(-k_f t)\right) + k_c M t$$

where

**$G_0^*$ ,  $M$  - Material dependent parameters**  
 **$A_f$ ,  $A_c$ ,  $E_{af}$ ,  $E_{ac}$  - Universal constants**

$M$  = binder aging rate,

$$k_f = A_f \exp\left(-E_{af}/RT\right),$$

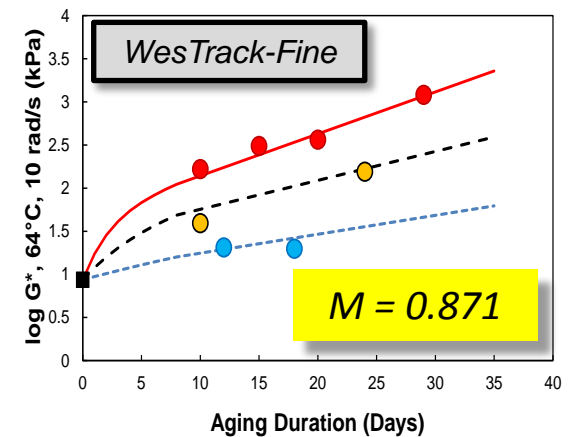
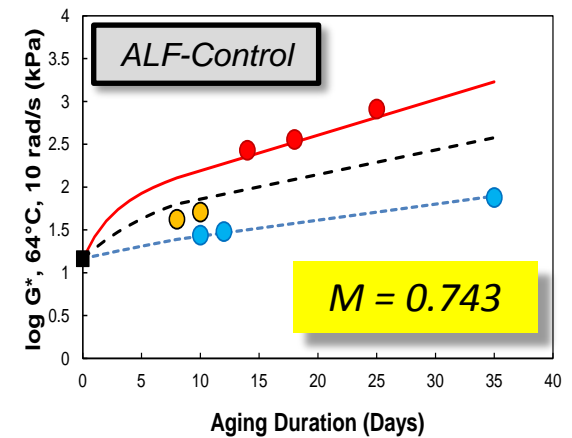
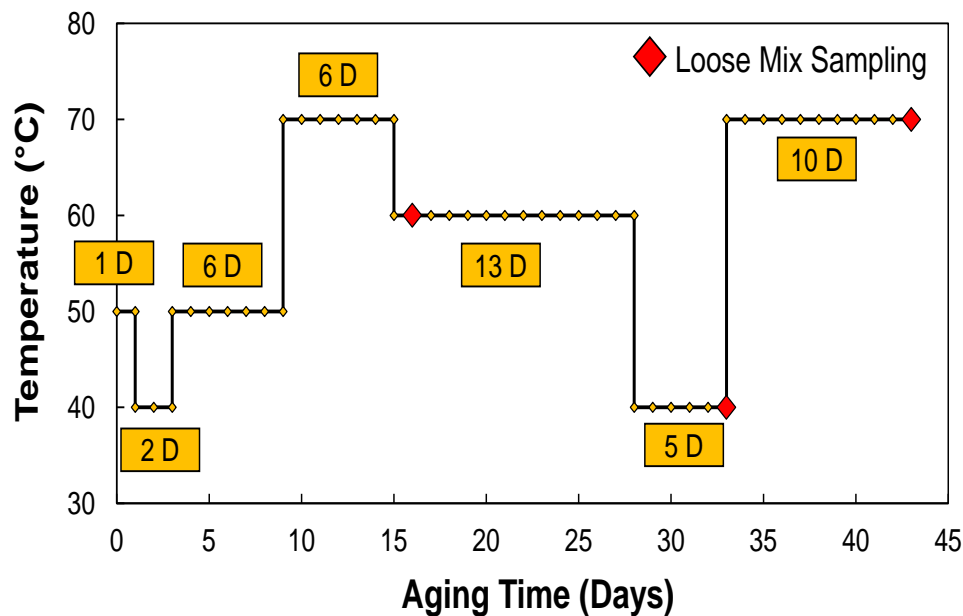
$$k_c = A_c \exp\left(-E_{ac}/RT\right), \text{ and}$$

$T$  = temperature in Kelvin.

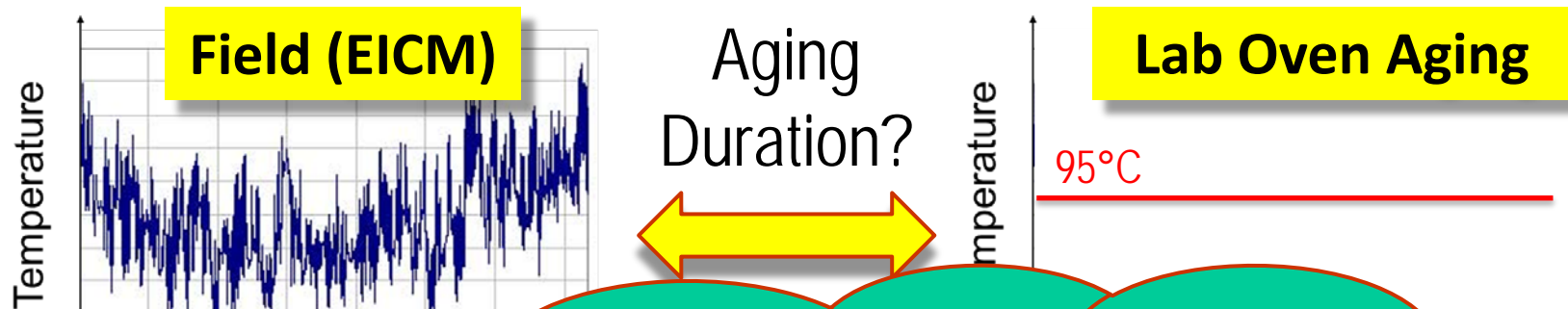
# Non-Isothermal Aging Verification

## Non-Isothermal Laboratory Aging Trial

- FHWA ALF Control
- WesTrack Fine 1995



# Kinetics Modeling to Find Required Duration to Match Field Aging



**Required Aging Duration to Match Field Aging is Independent of Binder Source/Type (i.e., STA G\* and M)**

$$\log G_{Lab}^* = \log a_0 \left[ \left(1 - \frac{k_c}{k_f}\right) (1 - \exp(-k_f t)) + k_c t \right]_{Lab}$$

$$\left[ \left(1 - \frac{k_c}{k_f}\right) (1 - \exp(-k_f t)) + k_c t \right]_{Field} = \left[ \left(1 - \frac{k_c}{k_f}\right) (1 - \exp(-k_f t)) + k_c t \right]_{Lab}$$



# Aging Durations Based on Climatic Data

# Climatic Aging Index (CAI)

$$CAI = \sum_{i=1}^{24} (D \times A \times \exp(-E_a/RT_i) / 24) = t_{oven}$$

$$D = \begin{cases} 3.4311 d^{-0.683} & \text{for } 6 \text{ mm} \leq d \leq 35 \text{ mm} \\ 0.3026 & \text{for } d > 35 \text{ mm} \end{cases}$$

where

$D$  = depth correction factor,

$A, E_a$  = fitting parameters,

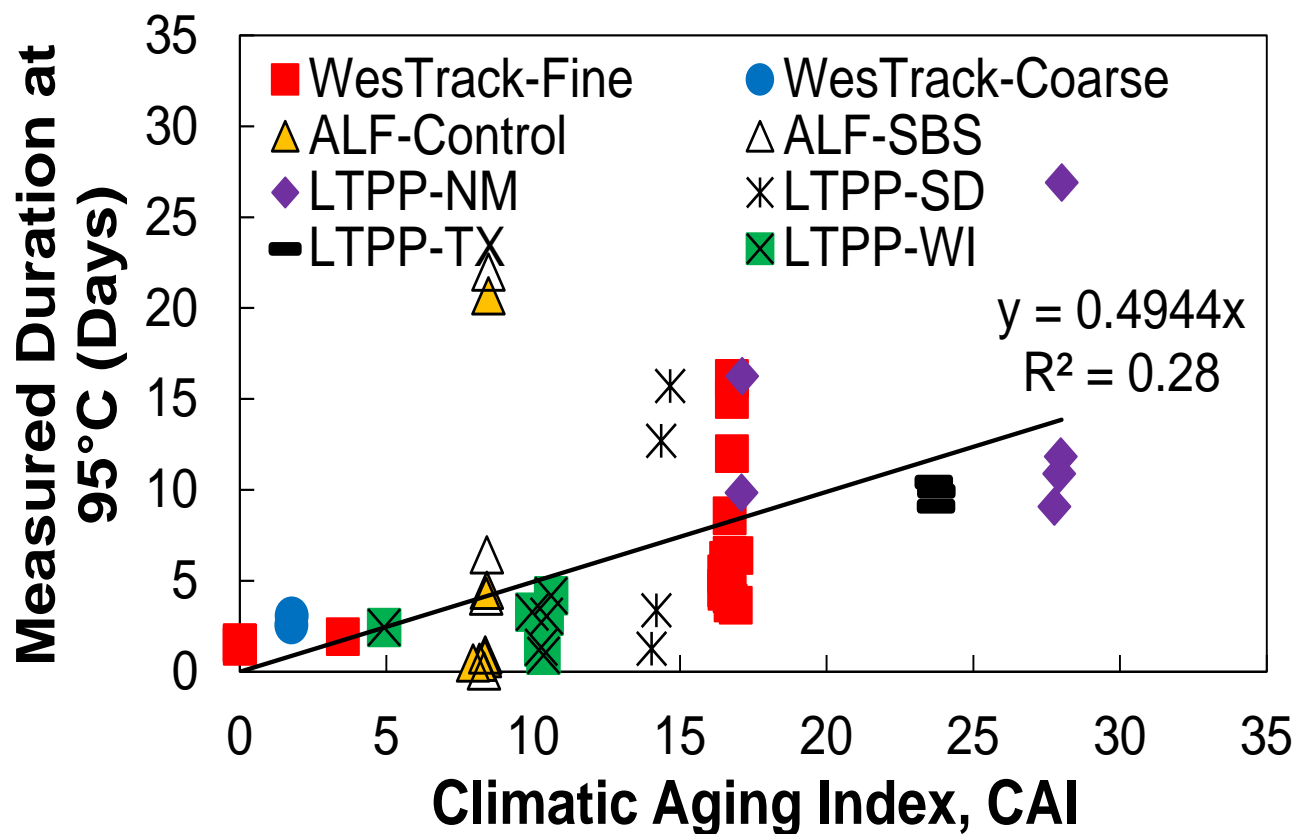
$R$  = universal gas constant,

$T$  = air temperature (Kelvin), and

$d$  = depth of interest (mm).

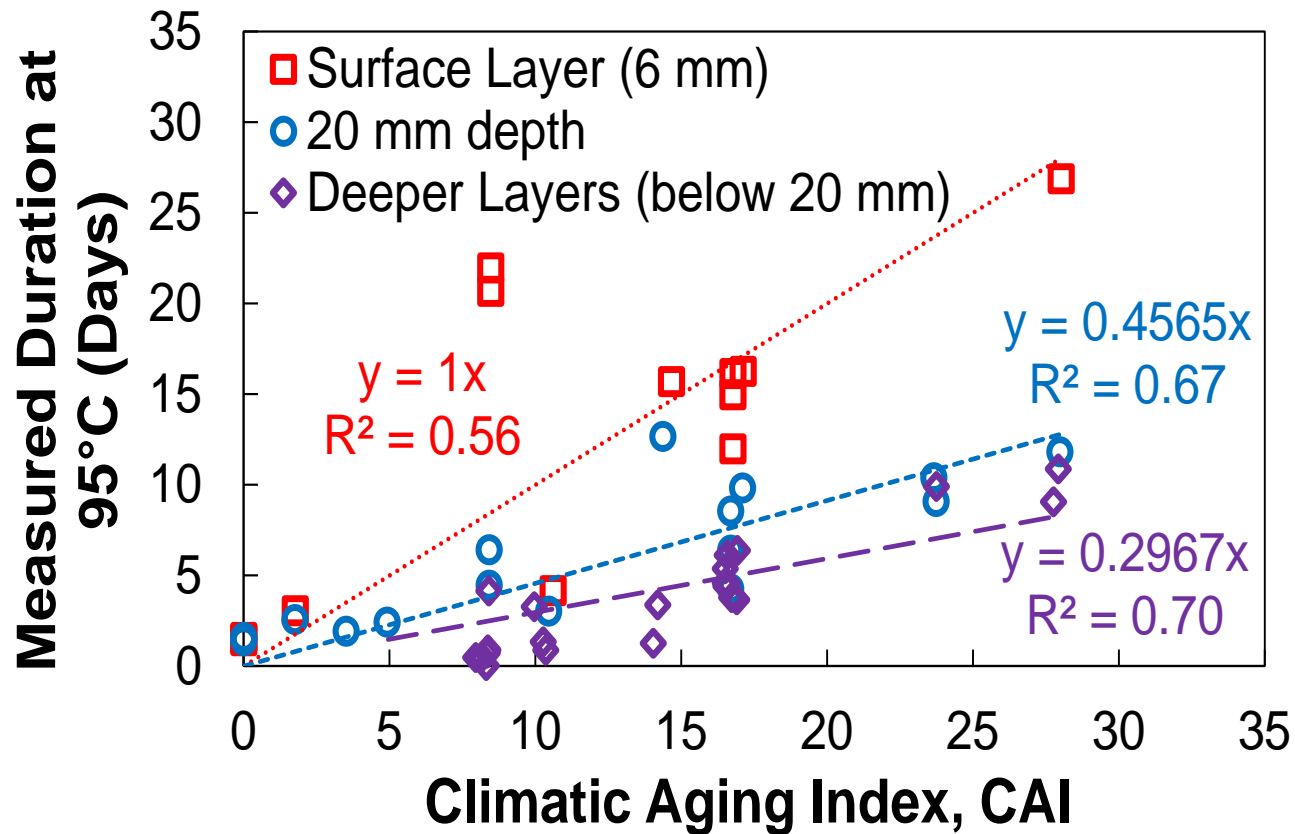
# Aging Durations Based on Climatic Data

*fitting without depth correction factor (D)*

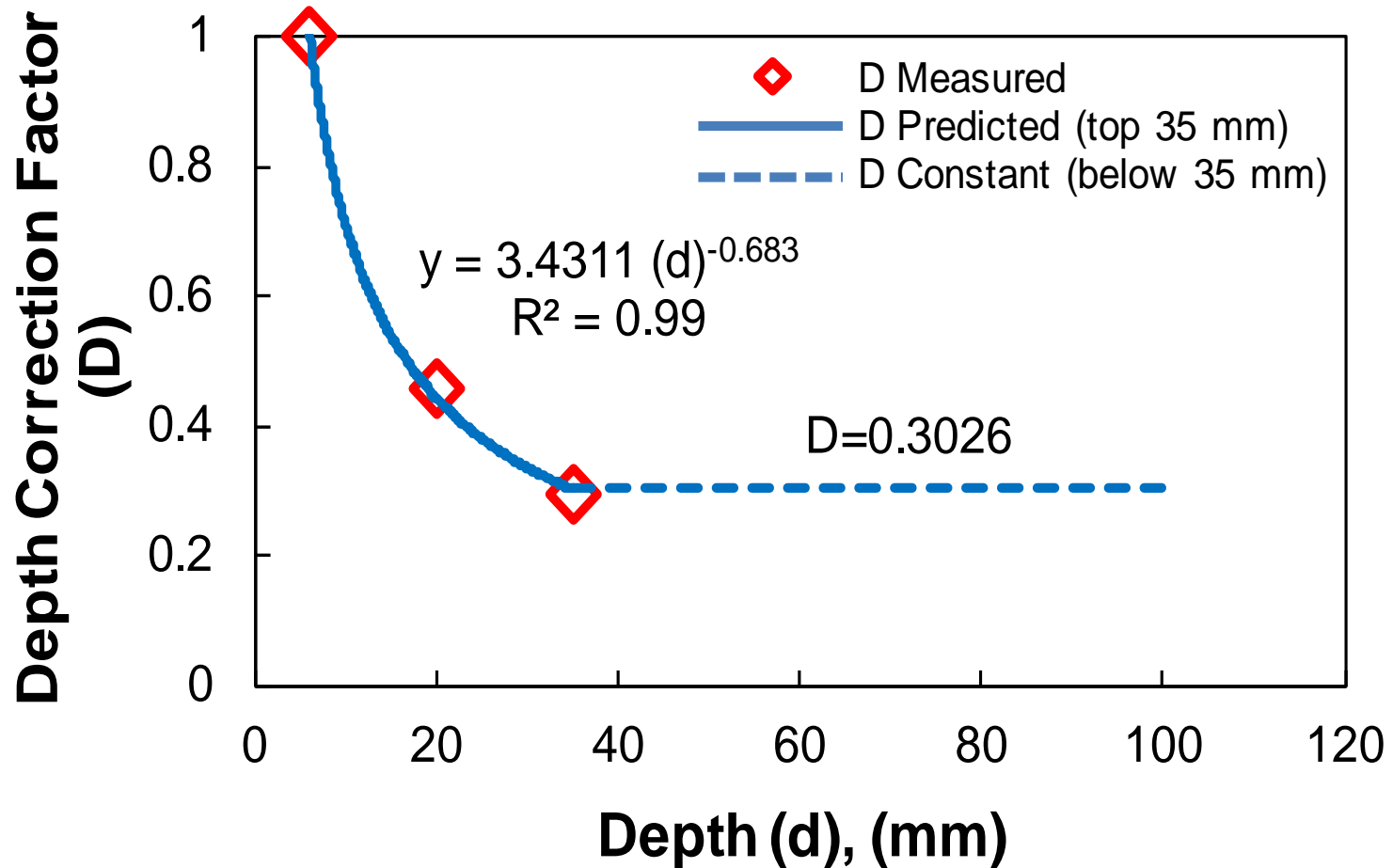


# Aging Durations Based on Climatic Data

*fitting for individual layer depths*

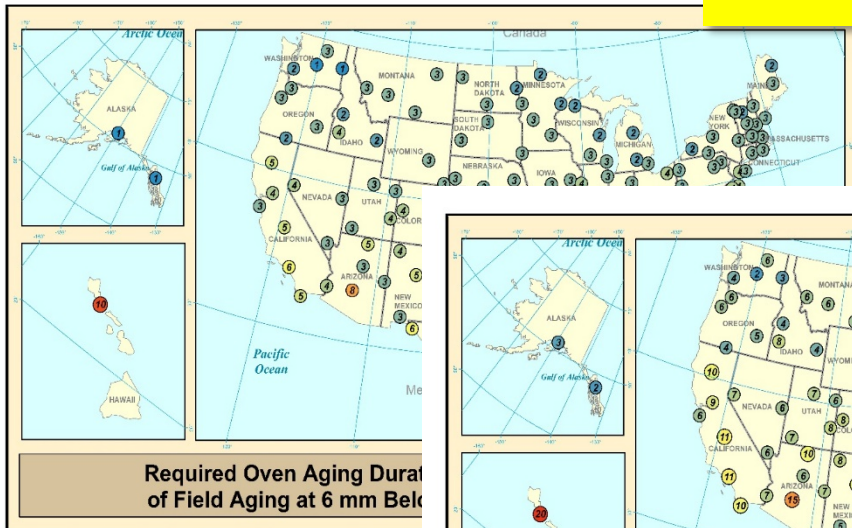


# Depth Correction Factor

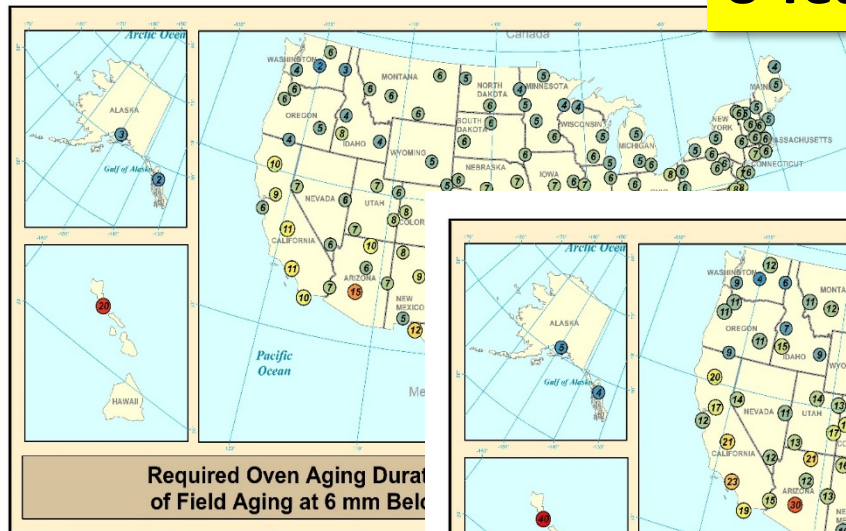


# Aging Duration Maps for 6 mm Depth

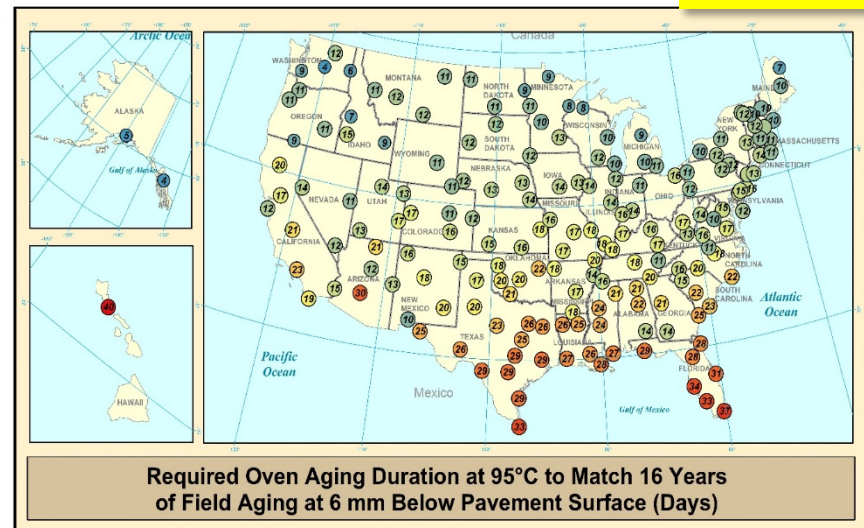
4 Years



8 Years

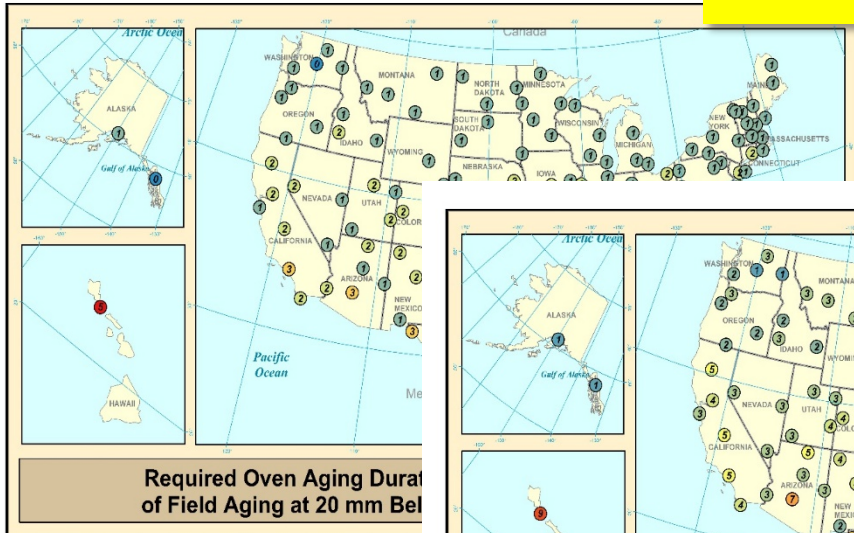


16 Years

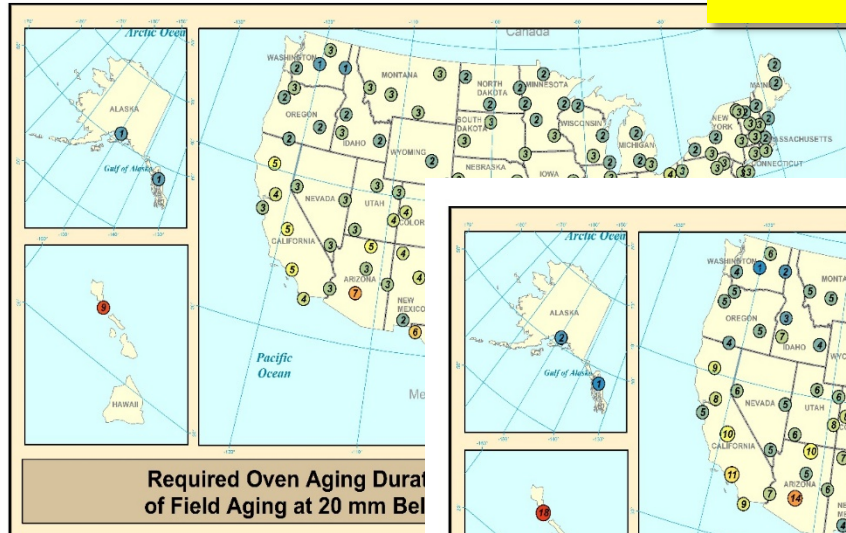


# Aging Duration Maps for 20 mm Depth

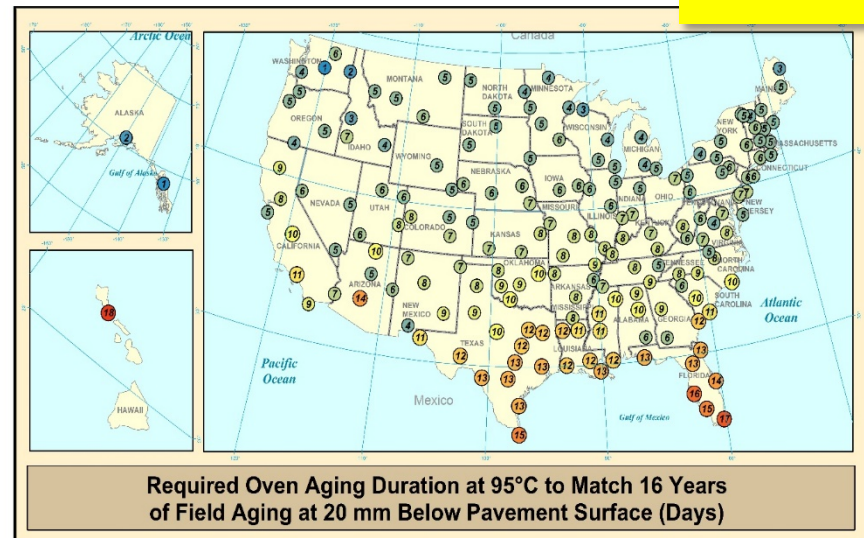
**4 Years**



**8 Years**

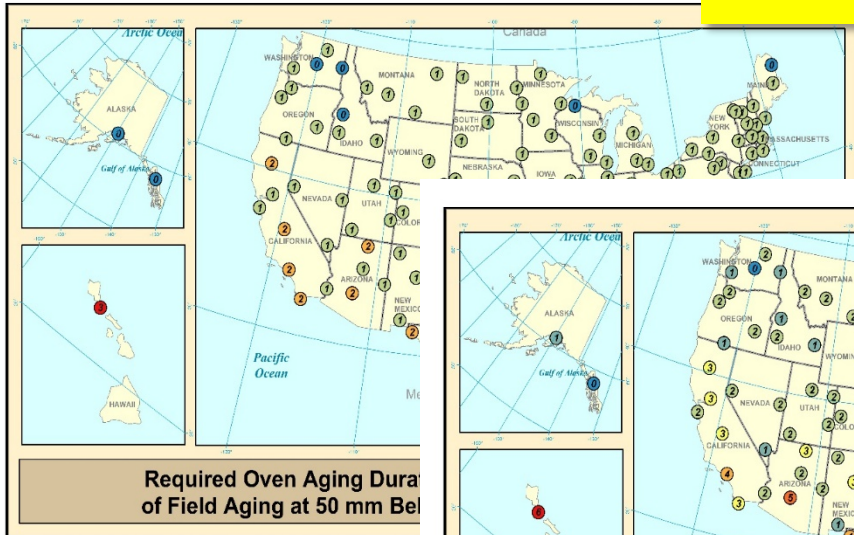


**16 Years**

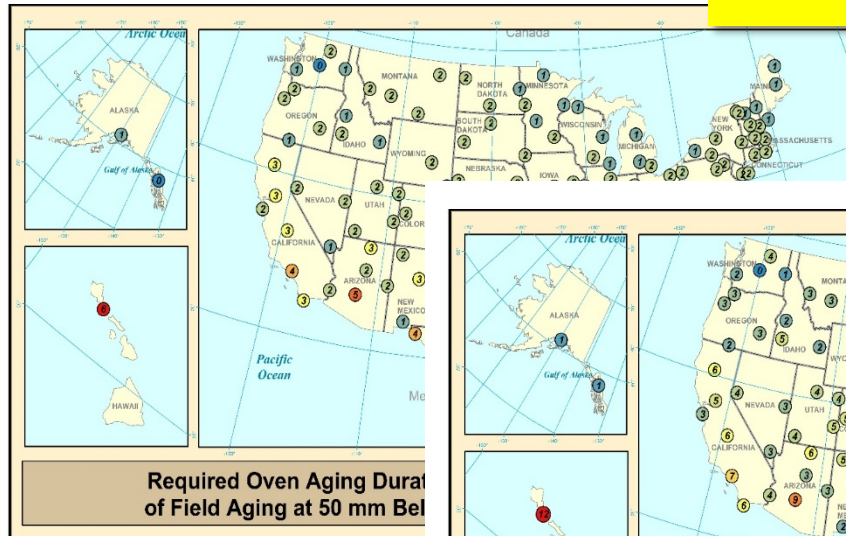


# Aging Duration Maps for 50 mm Depth

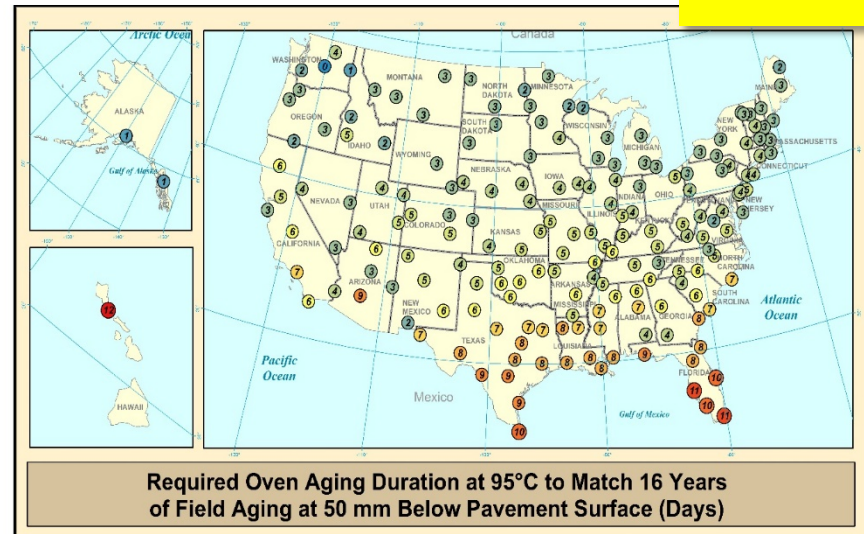
**4 Years**



**8 Years**



**16 Years**





# $G_{mm}$ in LTA Condition

## LTPP-South Dakota

Aging Duration (days)	$G_{mm}$
Short-Term Aged	2.419
Long-Term Aged (4 days)	2.440
Long-Term Aged (8 days)	2.444
Long-Term Aged (16 days)	2.446

## LTPP-Texas

Aging Duration (days)	$G_{mm}$
Short-Term Aged	2.428
Long-Term Aged (3 days)	2.442
Long-Term Aged (11 days)	2.446
Long-Term Aged (17 days)	2.448

# Proposed AASHTO Specification

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Standard Method of Test for

## **Long-Term Conditioning of Asphalt Mixture for Mixture Performance Testing**

AASHTO Designation: TP ###-##



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### **1. SCOPE**

- 1.1. This practice describes procedures for long-term conditioning of uncompacted asphalt mixture for mixture mechanical property testing to simulate the aging that occurs over the service life of a pavement. The long-term conditioning for mixture mechanical property testing procedure is preceded by the procedure for short-term conditioning for mixture mechanical property testing.

# Pavement Aging Model (Preliminary)

# Prediction of Field Aging Using Mix-Specific Kinetics Parameters

$$\log G^* = \log G_0^* + M \left(1 - \frac{k_c}{k_f}\right) (1 - \exp(-k_f t)) + k_c M t$$

*where*

$$k_f = A_f \exp(-E_{af}/RT) \quad k_c = A_c \exp(-E_{ac}/RT)$$

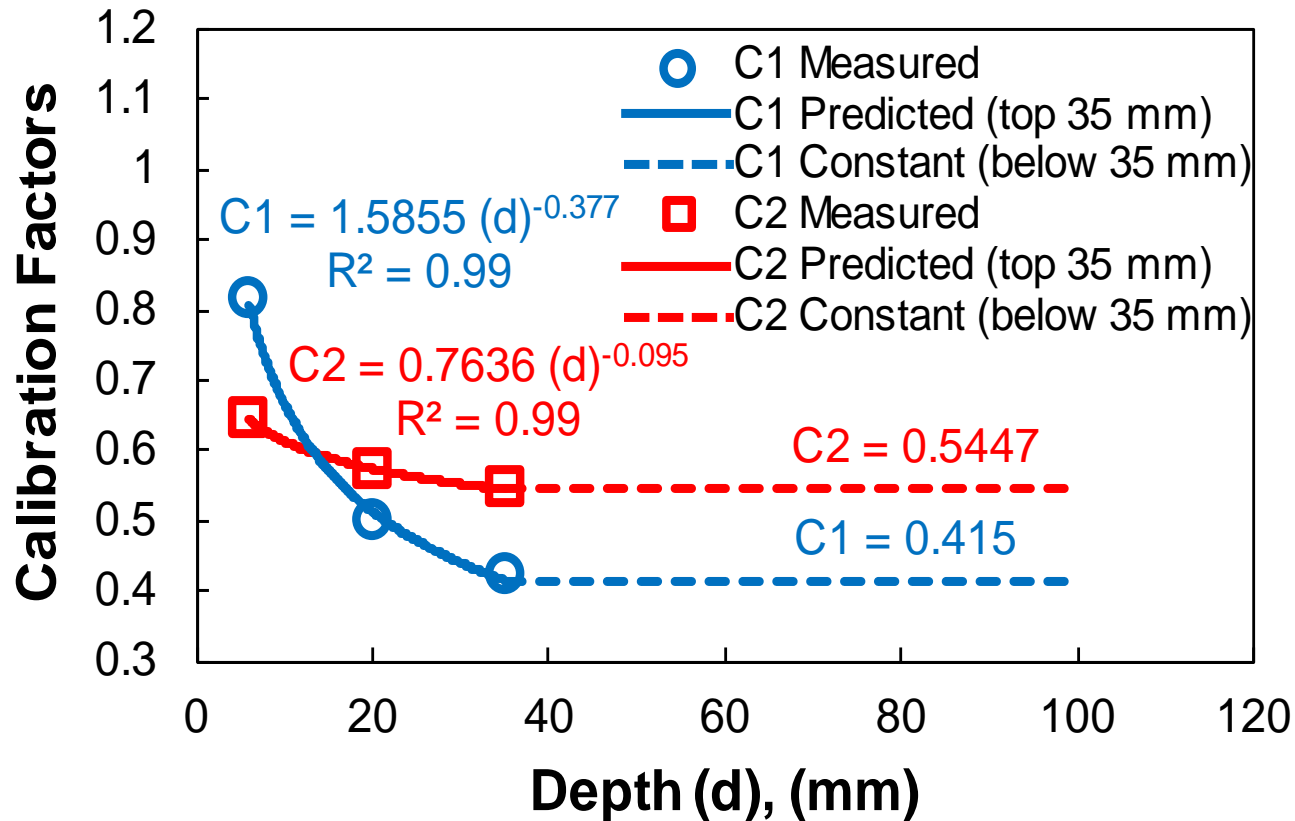
$$\log G^*_{field-measured} = C_1 + C_2 \times \log G^*_{field-predicted}$$

*where*

$C_1, C_2 =$  calibration factors

# Prediction of Field Aging Using Mix-Specific Kinetics Parameters

$$\log G^*_{field-measured} = C_1 + C_2 \times \log G^*_{field-predicted}$$



# Prediction of Field Aging Using Mix-Specific Kinetics Parameters

$$\log G^*_{field-measured} = C_1 + C_2 \times \log G^*_{field-predicted}$$

$$C_1 = \begin{cases} 1.5855 d^{-0.377}, & 6 \text{ mm} \leq d \leq 35 \text{ mm} \\ 0.415, & d > 35 \text{ mm} \end{cases}$$

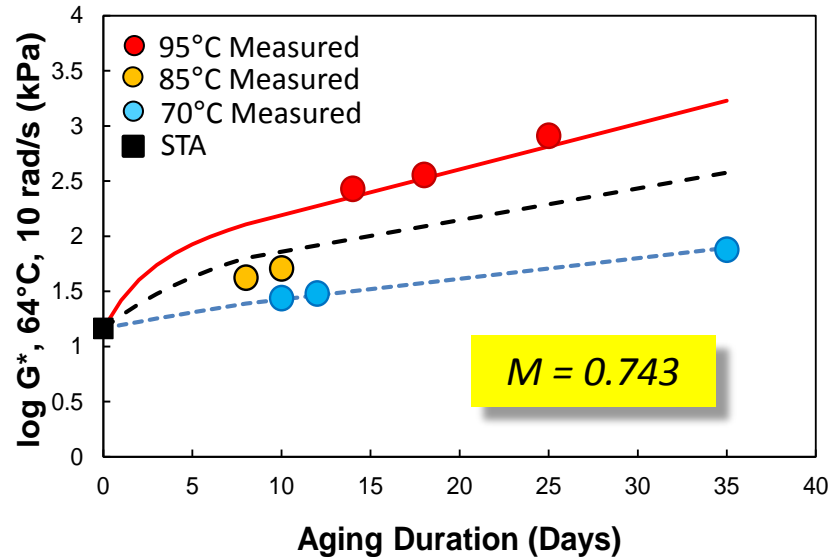
$$C_2 = \begin{cases} 0.7636 d^{-0.095}, & 6 \text{ mm} \leq d \leq 35 \text{ mm} \\ 0.5447, & d > 35 \text{ mm} \end{cases}$$

where

$C_1, C_2$  = calibration factors and  
 $d$  = depth of interest (mm).

# Prediction of Field Aging

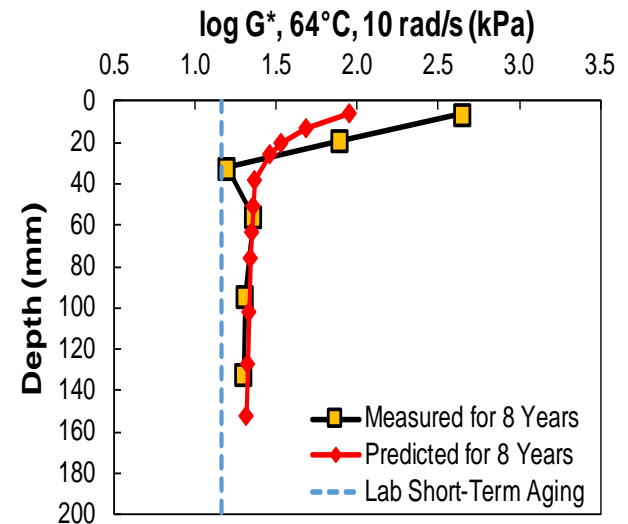
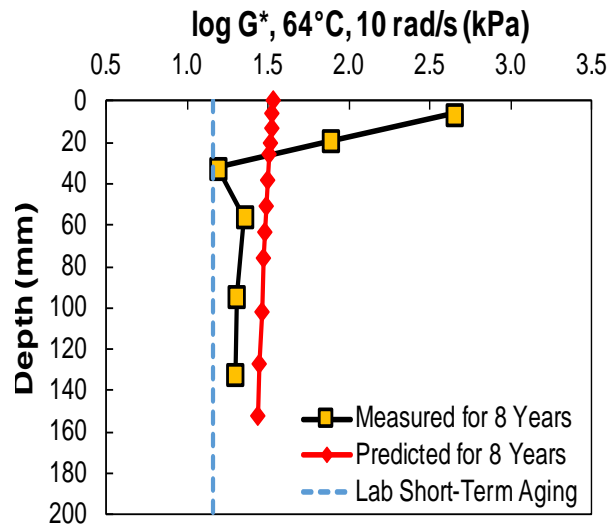
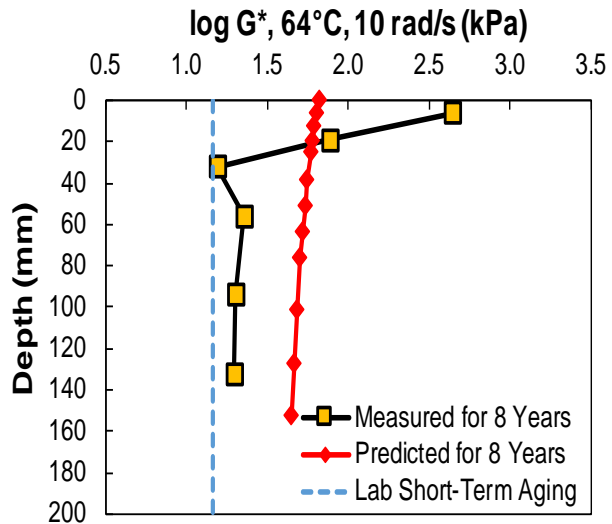
## ALF-Control (8 Years)



*Without Field Calibration*

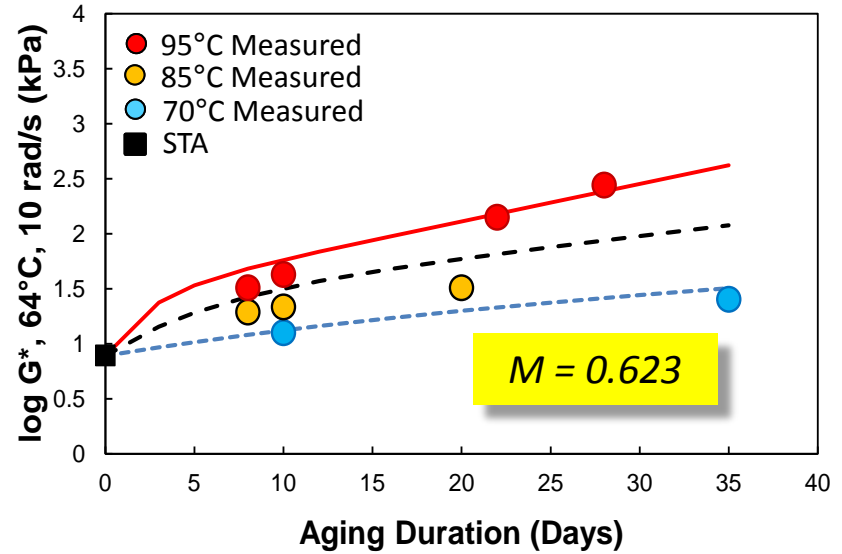
*20 mm Calibration Factor*

*Depth Dependent Calibration*



# Prediction of Field Aging

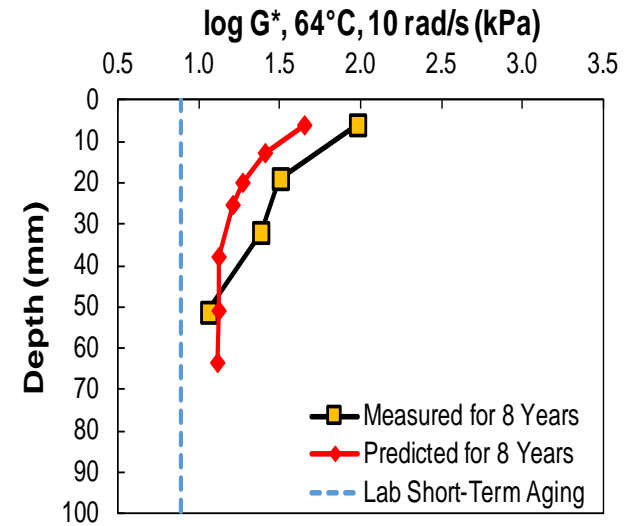
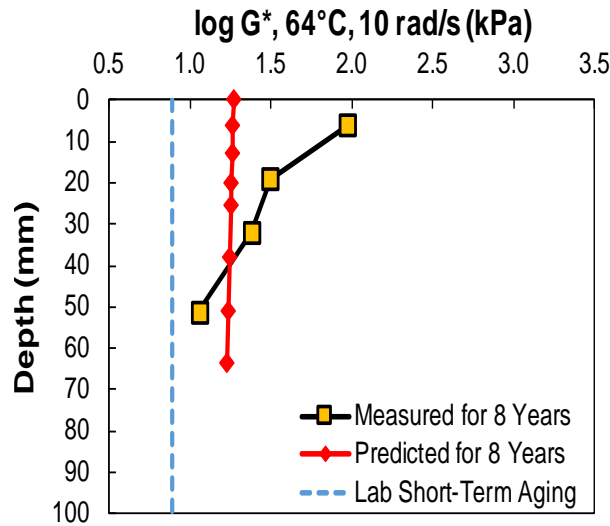
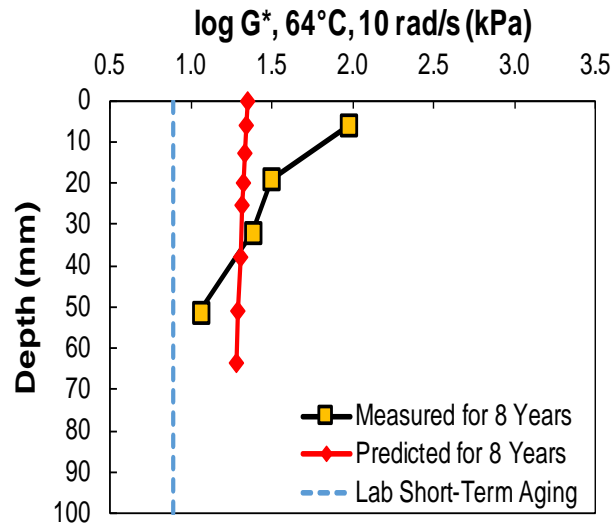
## ALF-SBS (8 Years)



*Without Field Calibration*

*20 mm Calibration Factor*

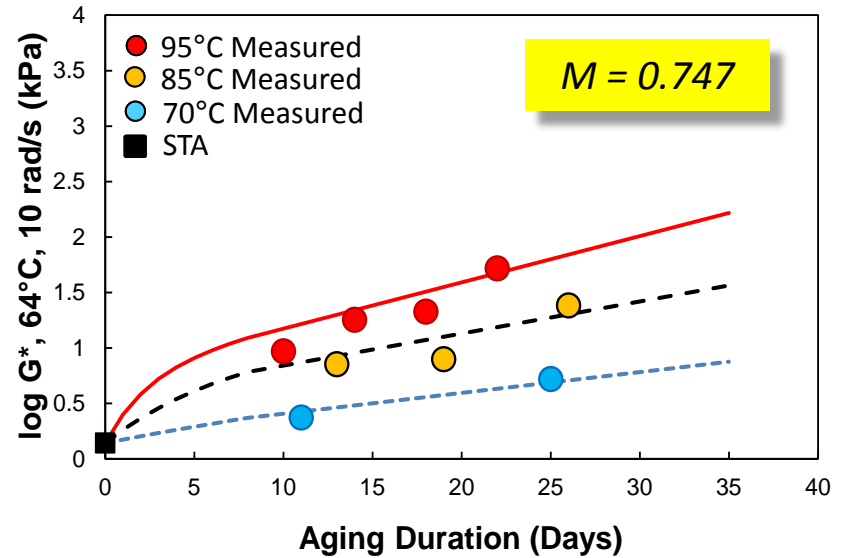
*Depth Dependent Calibration*





# Prediction of Field Aging

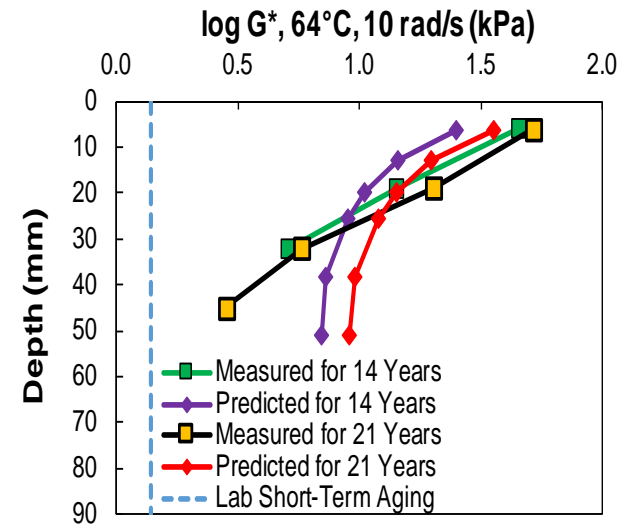
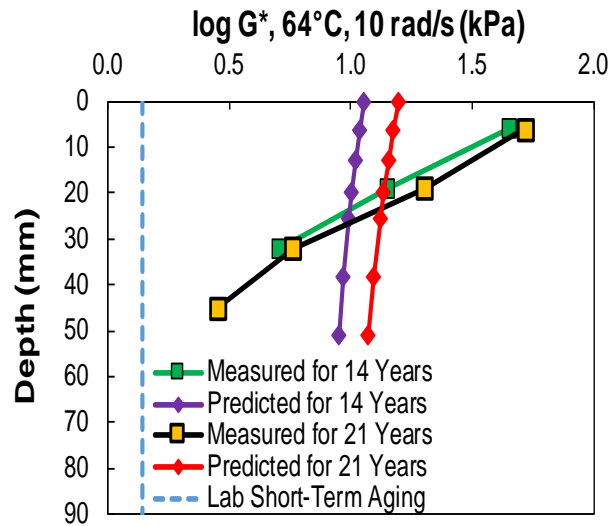
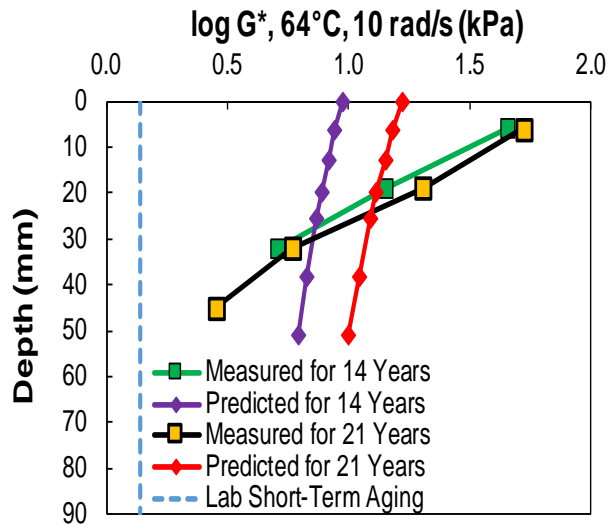
## LTPP-South Dakota (14 and 21 Years)



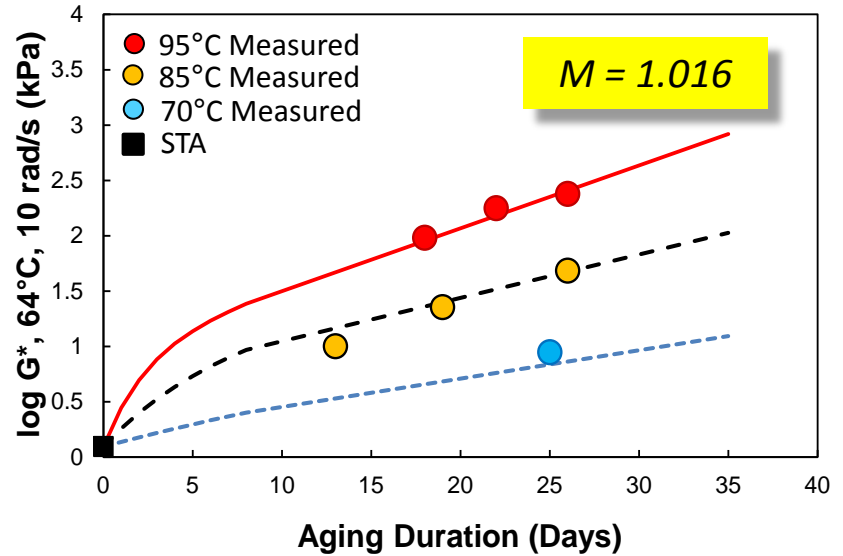
*Without Field Calibration*

*20 mm Calibration Factor*

*Depth Dependent Calibration*



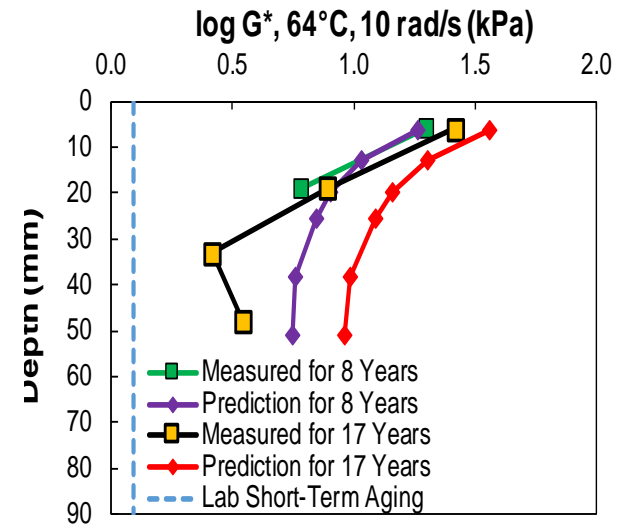
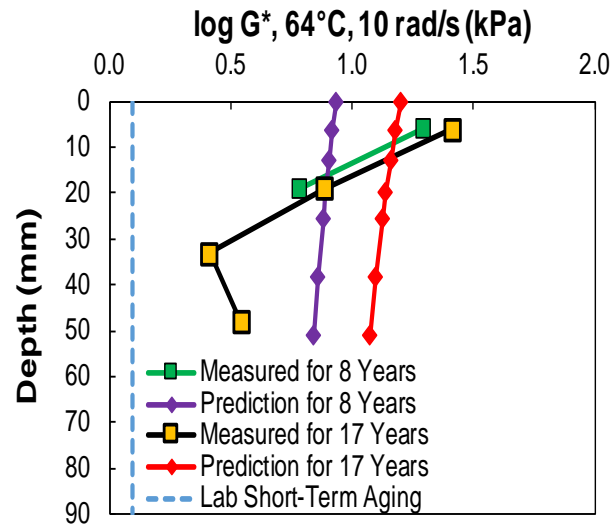
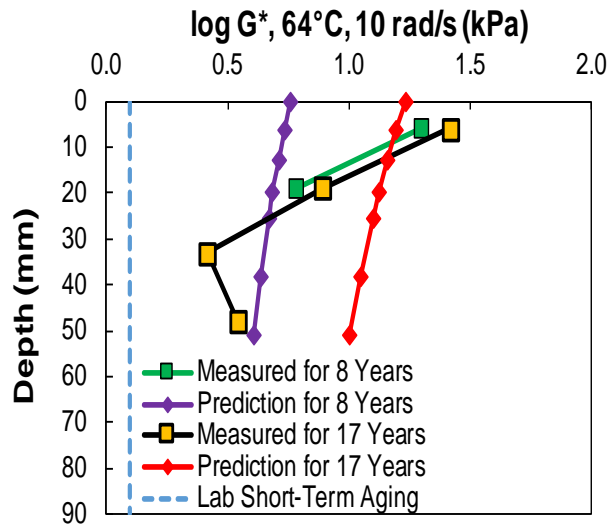
# Prediction of Field Aging LTPP-Wisconsin (8 and 17 Years)



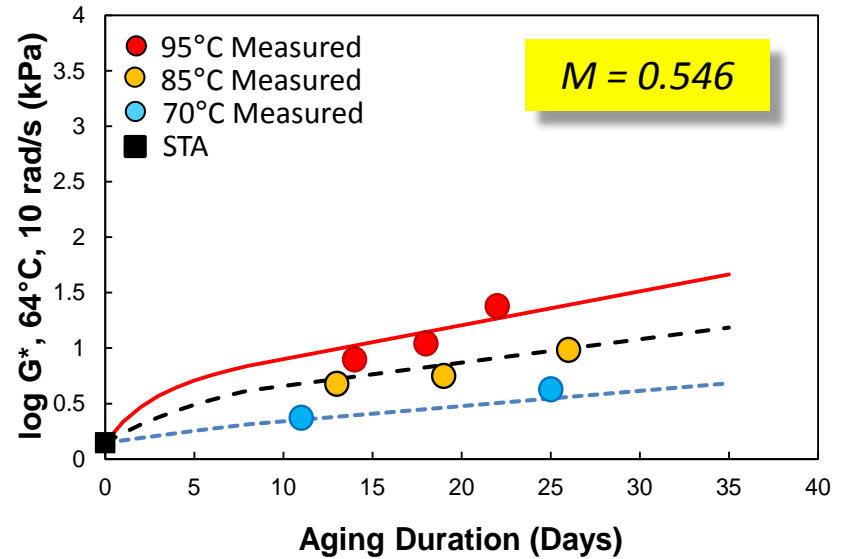
Without Field Calibration

20 mm Calibration Factor

Depth Dependent Calibration



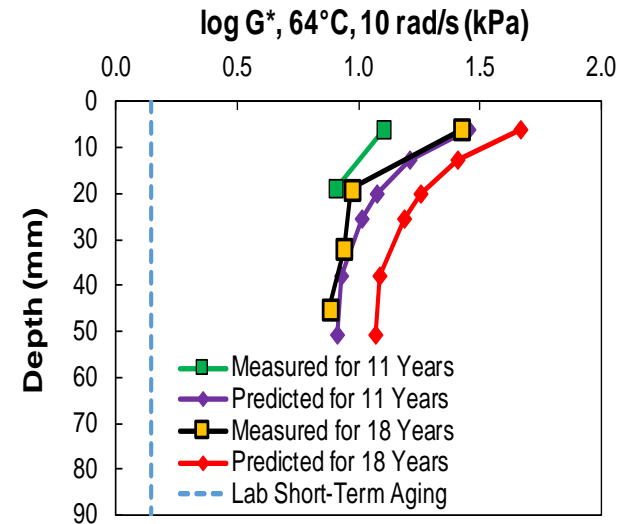
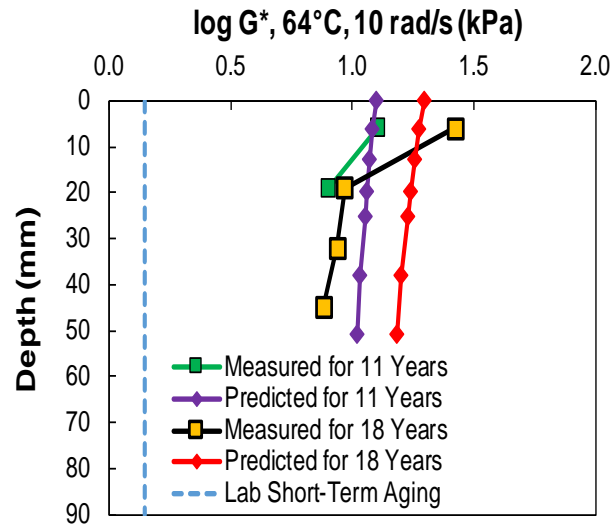
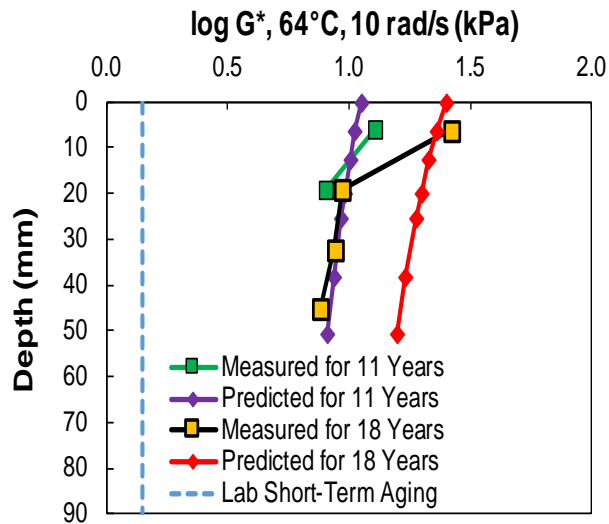
# Prediction of Field Aging LTPP-New Mexico (11 and 18 Years)



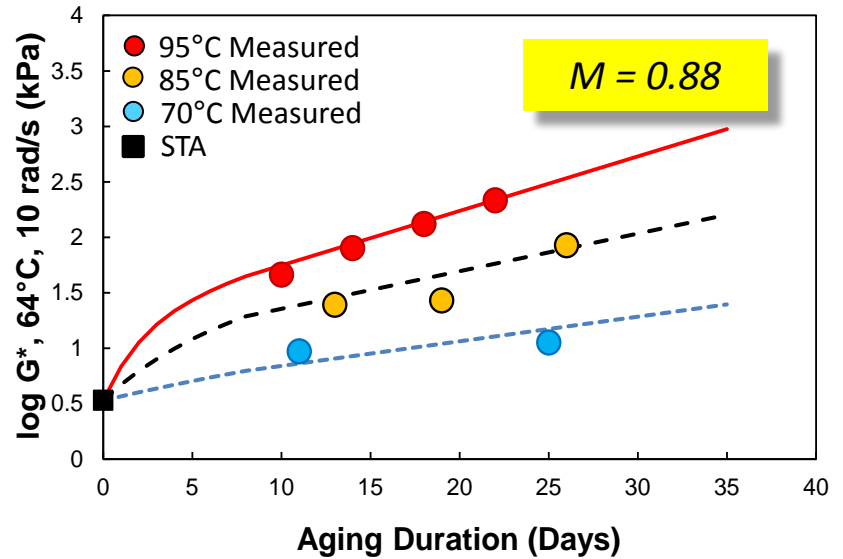
Without Field Calibration

20 mm Calibration Factor

Depth Dependent Calibration



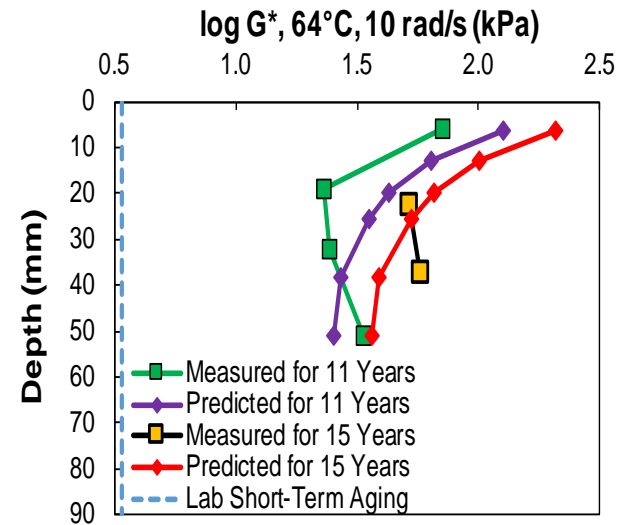
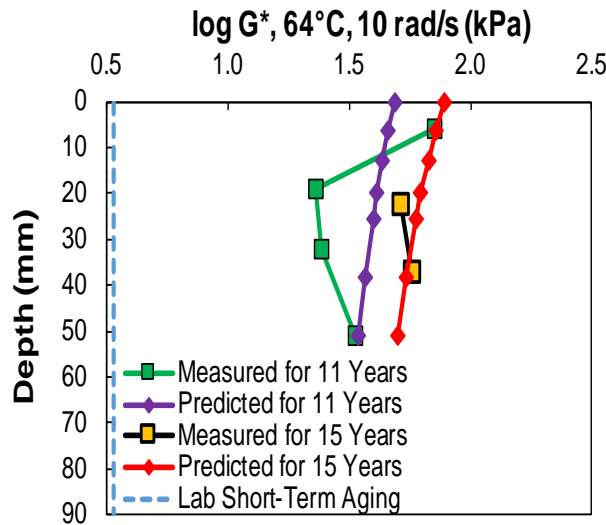
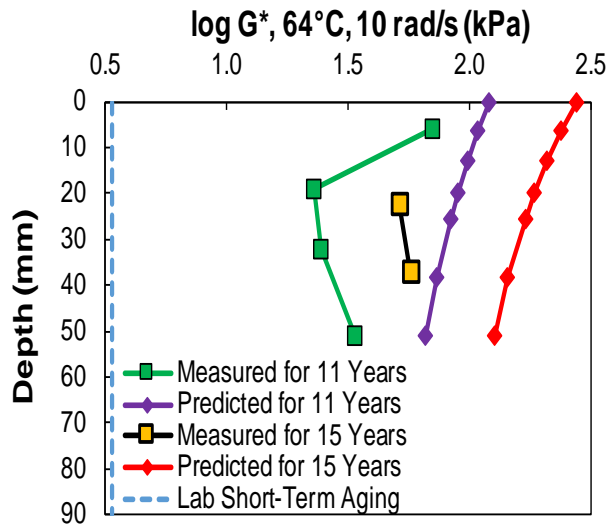
# Prediction of Field Aging LTPP-Texas (11 and 15 Years)



*Without Field Calibration*

*20 mm Calibration Factor*

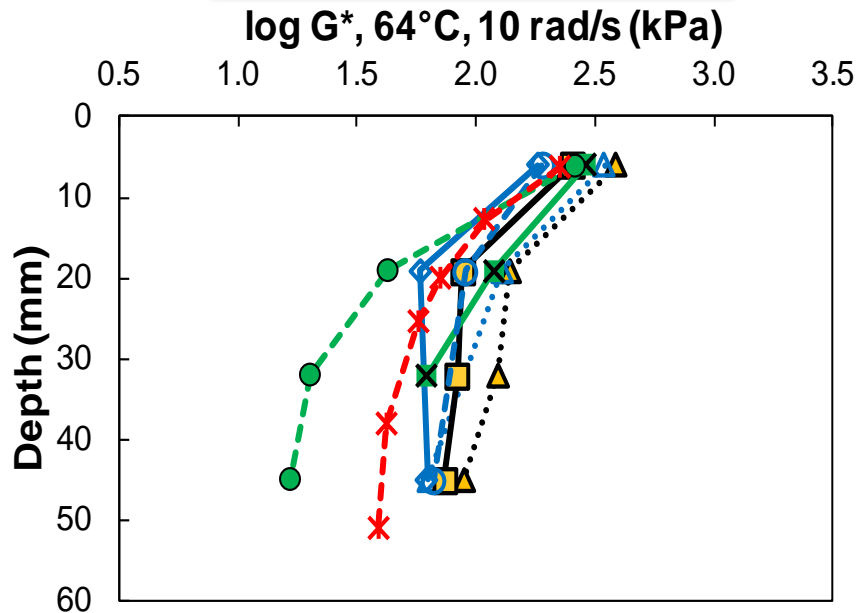
*Depth Dependent Calibration*



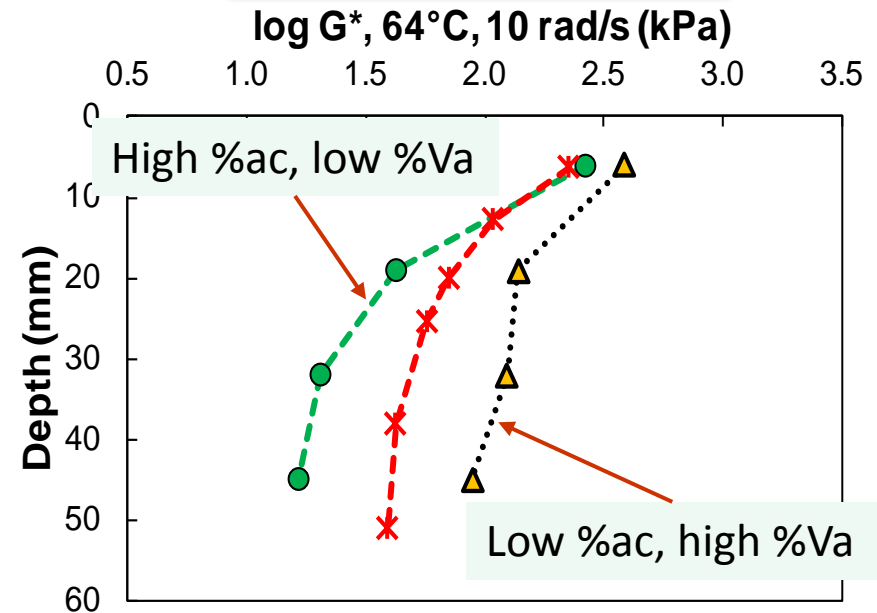
# Diffusion Effect WesTrack Fine Sections (19 years)

- \* Predicted log G\* for 19 Years of Field Aging
- ▲ Measured log G\* for Low %ac and High %Va
- ▴ Measured log G\* for Low %ac and Medium %Va
- Measured log G\* for Optimum %ac and High %Va
- ◊ Measured log G\* for Optimum %ac and Medium %Va
- ⊠ Measured log G\* for Optimum %ac and Low %Va
- Measured log G\* for High %ac and Medium %Va
- Measured log G\* for High %ac and Low %Va

## All Seven Sections



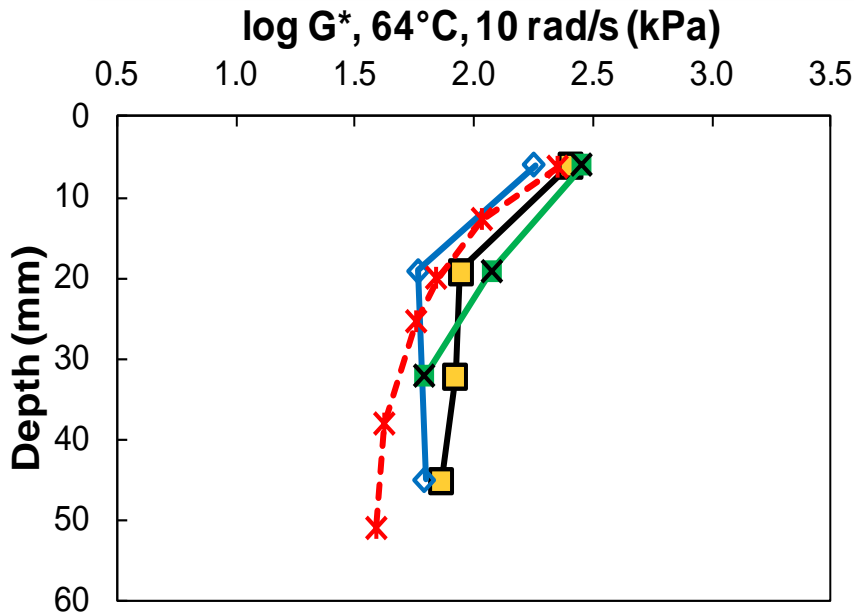
## Extreme Sections



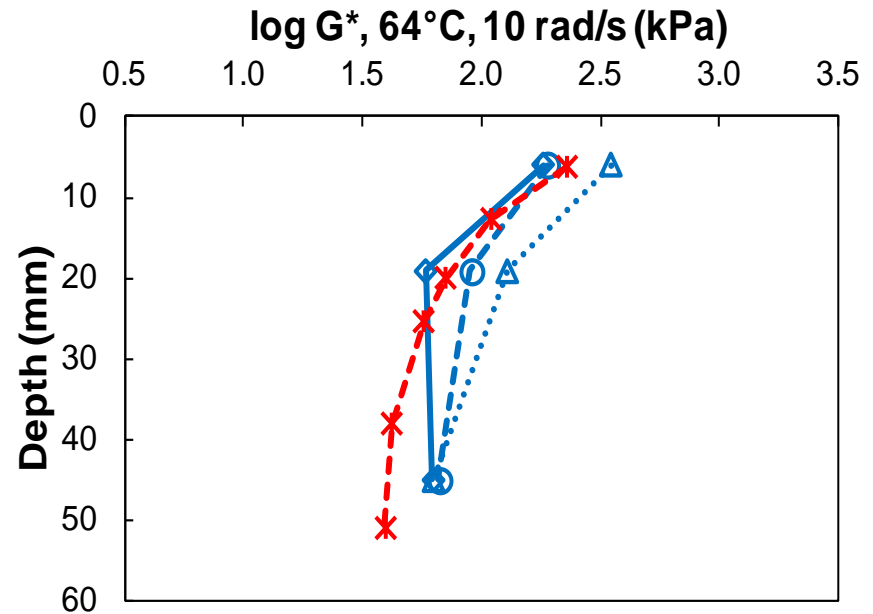
# Diffusion Effect WesTrack Fine Sections (19 years)

- \* Predicted log G\* for 19 Years of Field Aging
- ▲ Measured log G\* for Low %ac and High %Va
- △ Measured log G\* for Low %ac and Medium %Va
- Measured log G\* for Optimum %ac and High %Va
- ◆ Measured log G\* for Optimum %ac and Medium %Va
- ⊠ Measured log G\* for Optimum %ac and Low %Va
- Measured log G\* for High %ac and Medium %Va
- Measured log G\* for High %ac and Low %Va

**Optimum %ac, Varying %Va**



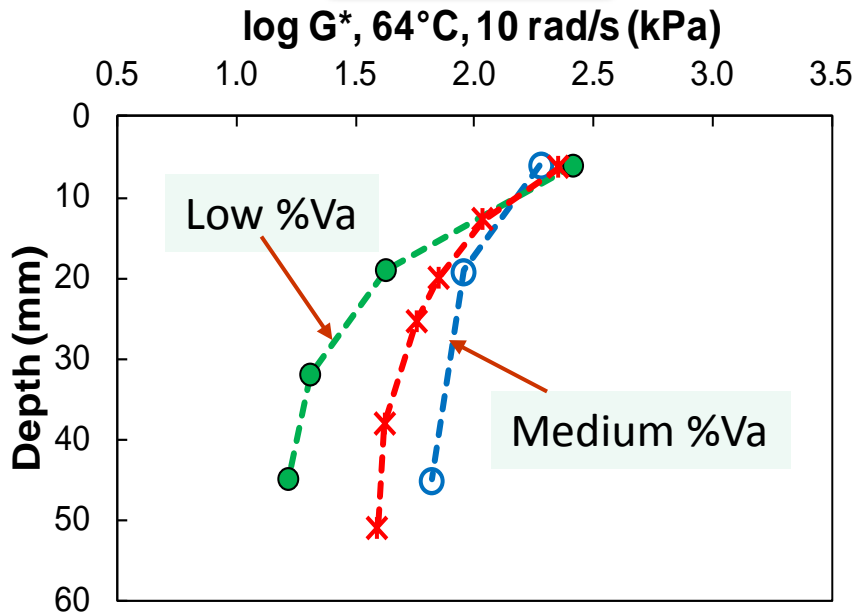
**Medium %Va, Varying %ac**



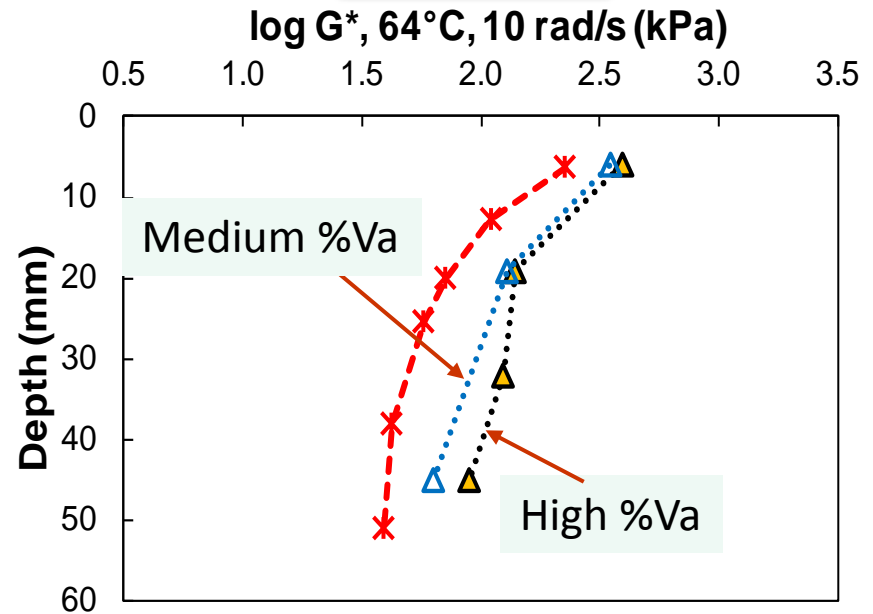
# Diffusion Effect WesTrack Fine Sections (19 years)

- \* Predicted log G\* for 19 Years of Field Aging
- ▲ Measured log G\* for Low %ac and High %Va
- △ Measured log G\* for Low %ac and Medium %Va
- Measured log G\* for Optimum %ac and High %Va
- ◇ Measured log G\* for Optimum %ac and Medium %Va
- ▣ Measured log G\* for Optimum %ac and Low %Va
- Measured log G\* for High %ac and Medium %Va
- Measured log G\* for High %ac and Low %Va

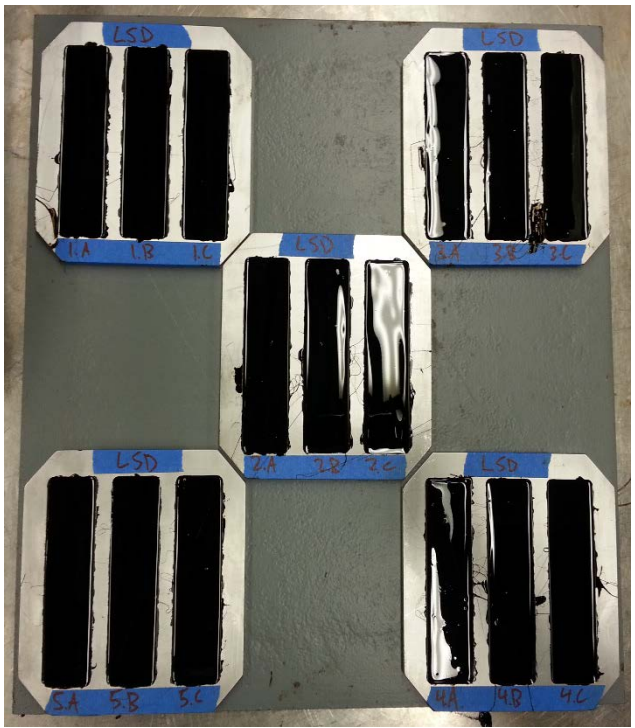
**High %ac**



**Low %ac**



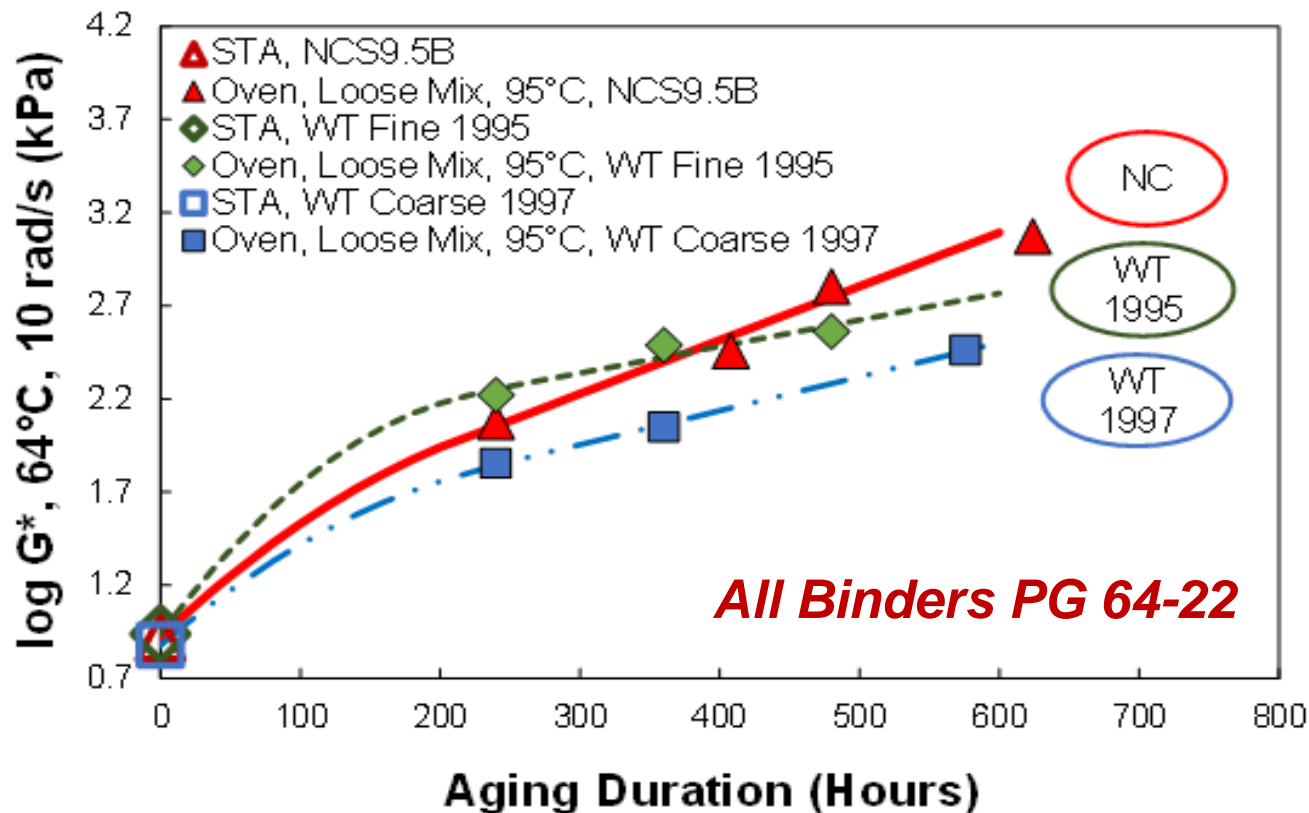
# Binder USAT Aging in Place of Loose Mix Aging





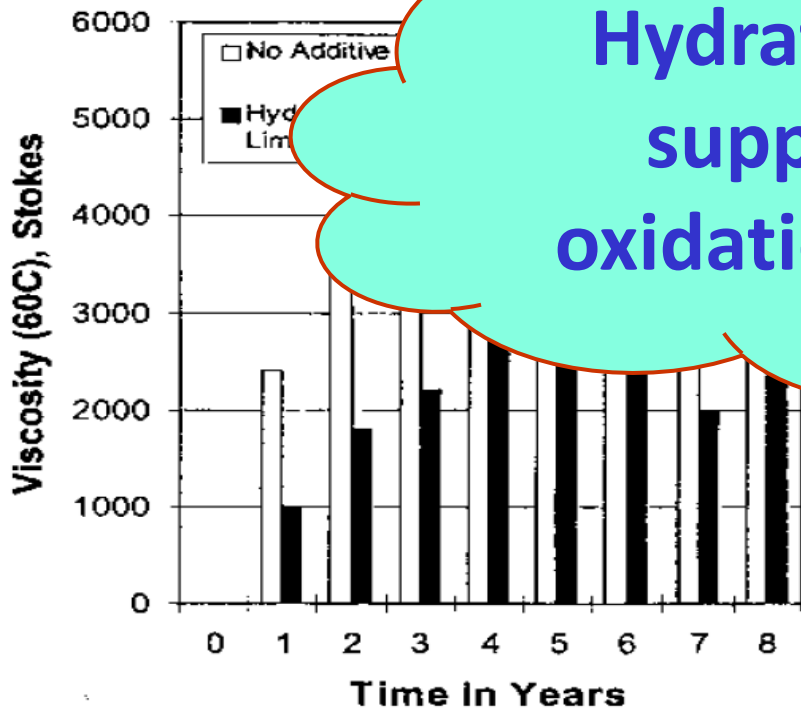
# Motivation for Binder-Specific Aging

- Example of binders with similar PG grades and quite different oxidation rates



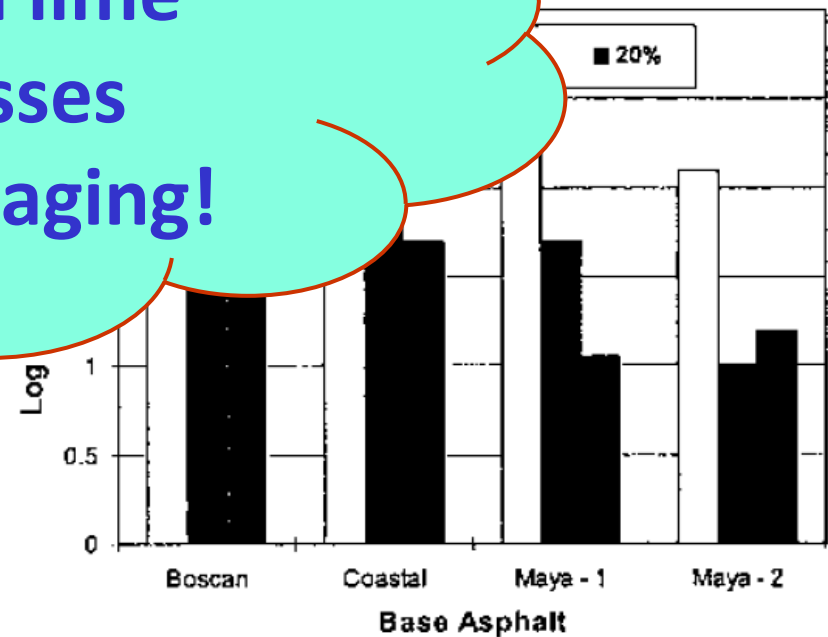
# Effect of Hydrated Lime

Field Data Demonstrating the Effect of Hydrated Lime on the Hardening of Asphalt Binder based on Utah Data (after Jones)



(Jones 1997)

Effect of Hydrated Lime in Reducing Aging Index of Asphalt Binders (after Peterson et al.)



(Petersen et al. 1987)

Hydrated lime suppresses oxidation aging!

# Study Mixture Details

	Hydrated Lime	Aggregate Type	%Pass Sieve #200 (P <sub>200</sub> )	Binder Content	Binder Source	Binder Grade	P <sub>200</sub> to Binder Content Ratio
<b>FHWA ALF - Control</b>	✓ (1%)	Limestone/ Diabase (Traprock)	6.3%	5.3%	Citgo-Venezuelan Bachaquero	PG 70-22	1.19
<b>WesTrack - Coarse</b>	✓ (1.5%)	Crushed Andesite/ Sand	6.5%	5.7%	Idaho Asphalt	PG 64-22	1.14
<b>WesTrack - Fine</b>	✓ (1.5%)	Andesite/ Granite/ Sand	5.4%	5.4%	West Coast Refinery	PG 64-22	1
<b>Validation</b>							
<b>SHRP-AAD</b>	✓ (1%)	Limestone/ Diabase (Traprock)	6.3%	5.3%	CA Coast	PG 58-28	1.19
<b>SHRP-AAG</b>	✓ (1%)	Limestone/ Diabase (Traprock)	6.3%	5.3%	CA Valley	PG 58-10	1.19

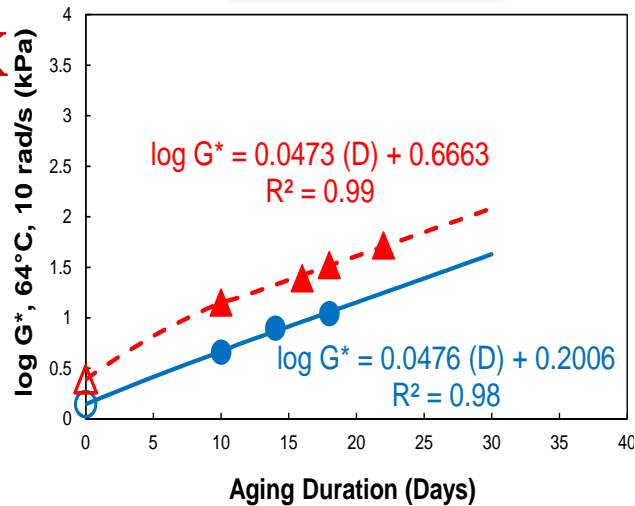
# Mixture Details

	Hydrated Lime	Aggregate Type	%Pass Sieve #200 (P <sub>200</sub> )	Binder Content	Binder Source	Binder Grade	P <sub>200</sub> to Binder Content Ratio
LTPP - NM	None	N/A	6.1%	7.6%	N/A	AC-20	0.8
LTPP - SD	None	N/A	4.6%	5.9%	N/A	120-150 Pen	0.78
LTPP - TX	None	N/A	5.7%	5.4%	N/A	AC-20	1.06
LTPP-WI	None	N/A	4.1%	5.9%	N/A	N/A	0.69
<b>Validation</b>							
NC	None	Granite	5.7%	6.6%	Citgo – Wilmington, NC	PG 64-22	0.86

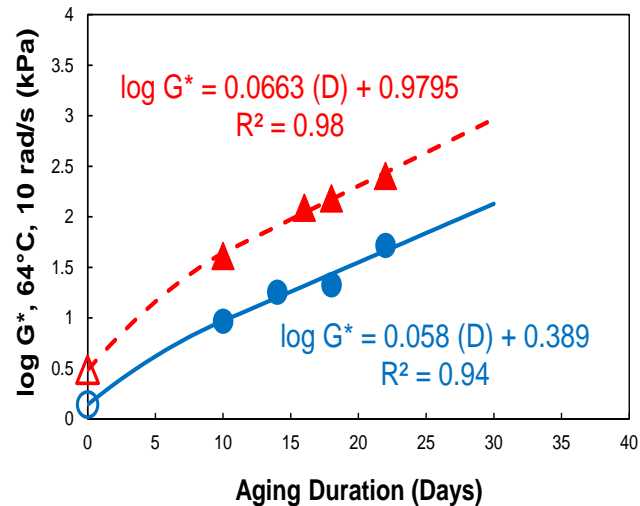
# Binder USAT and Loose Mix Aging Rates No Lime

- Loose Mix Aging
- ▲ Binder USAT Aging

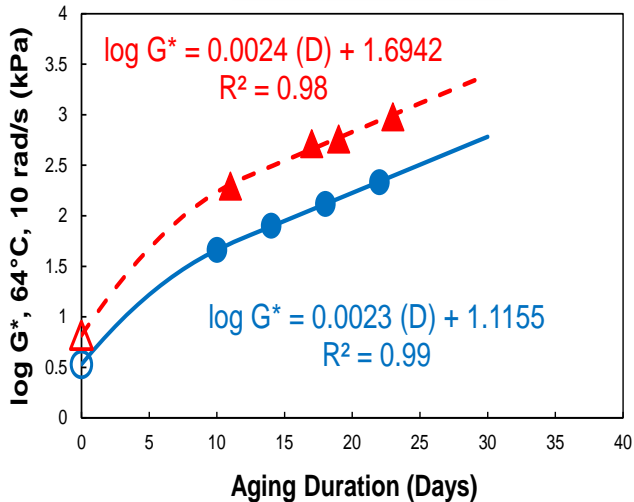
LTPP-NM



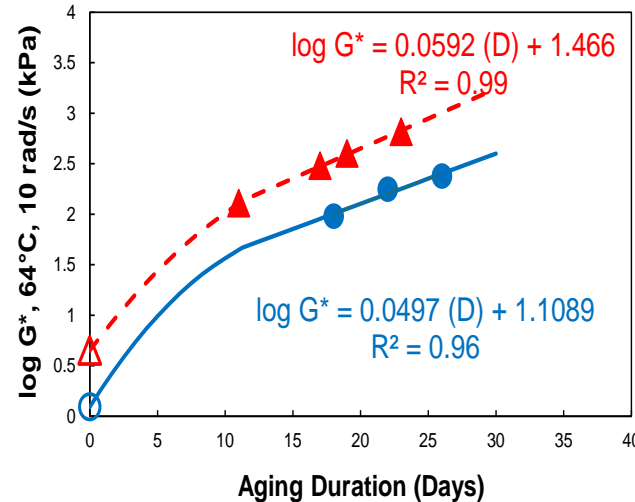
LTPP-SD



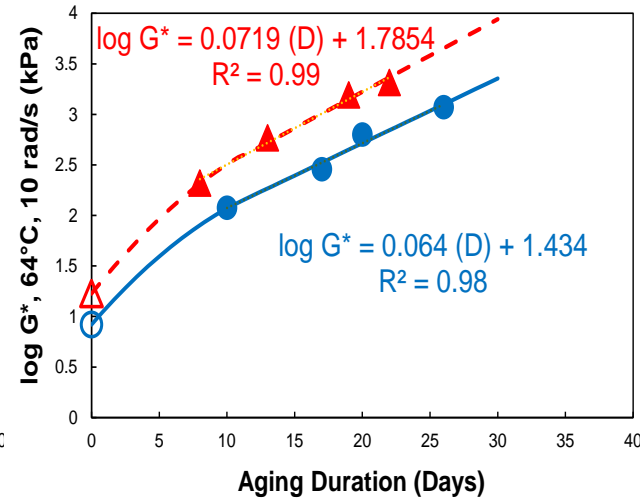
LTPP-TX



LTPP-WI



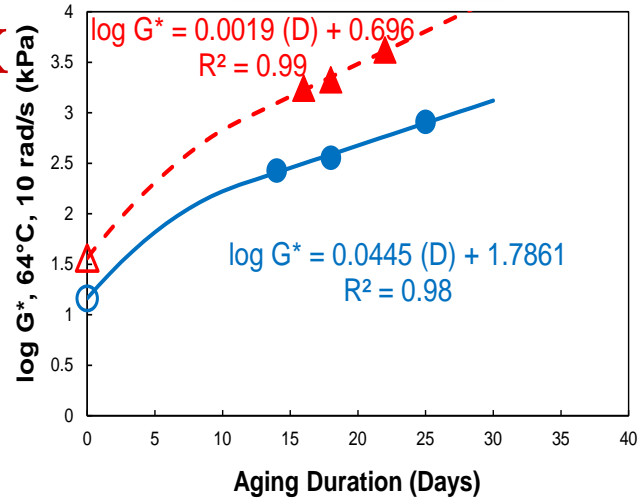
NC S9.5B



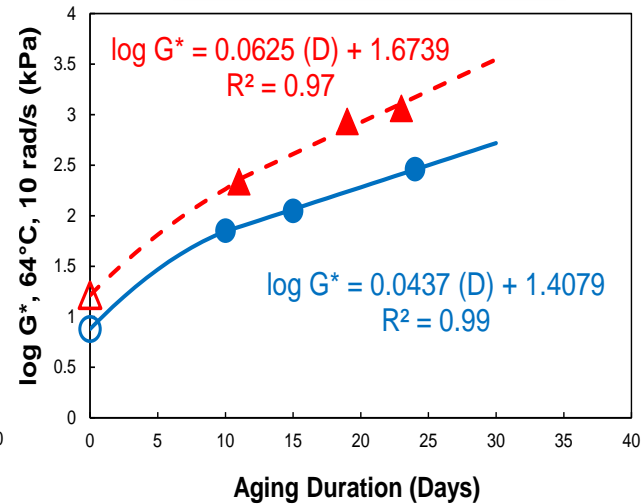
# Binder USAT and Loose Mix Aging Rates With Lime

- Loose Mix Aging
- ▲ Binder USAT Aging

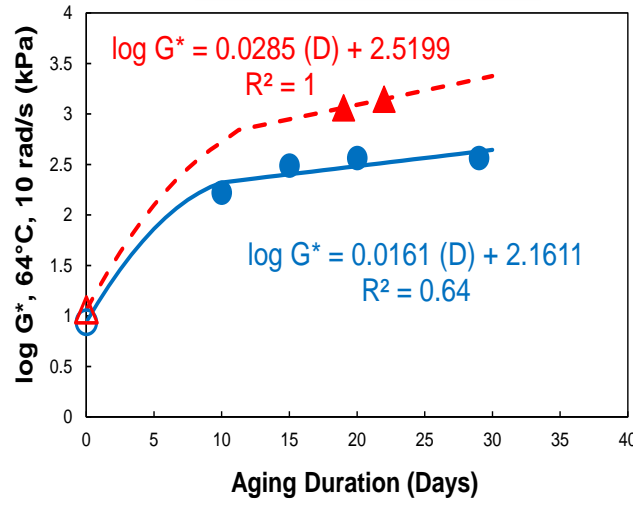
*ALF-Control*



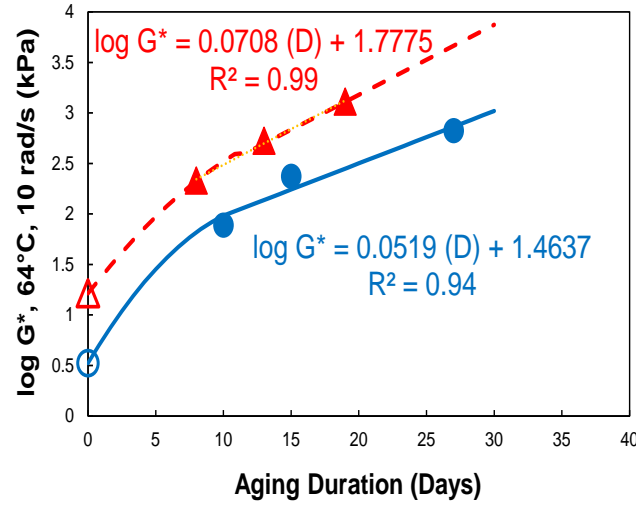
*WesTrack-Coarse*



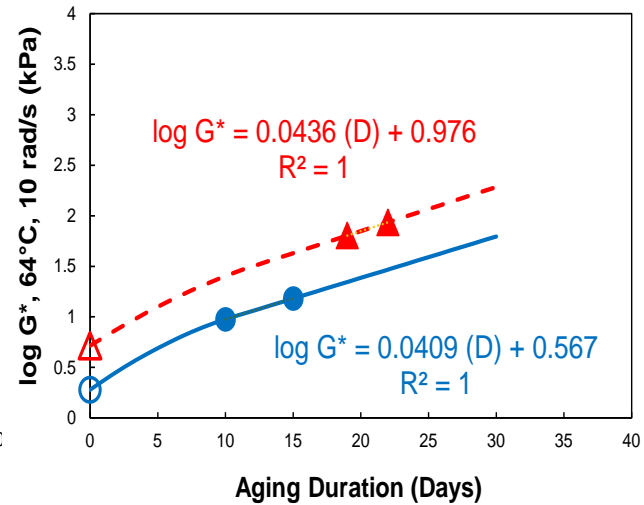
*WesTrack-Fine*



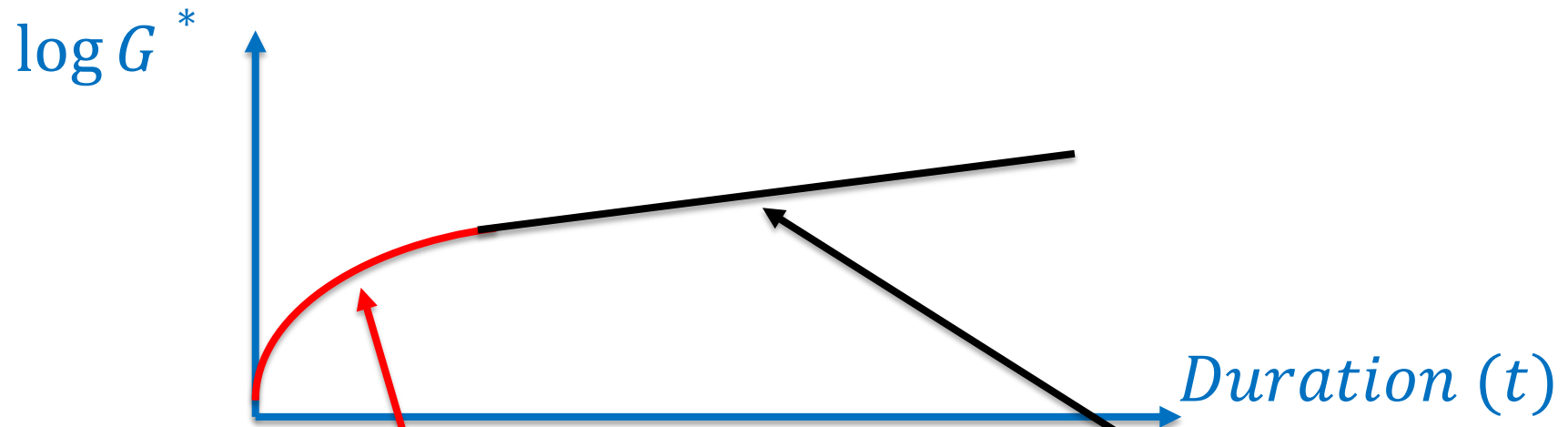
*SHRP AAD-1*



*SHRP AAG-1*



# Model Parameters



*Fast Rate*

$$\log G^* = a(t)^2 + b(t) + c$$

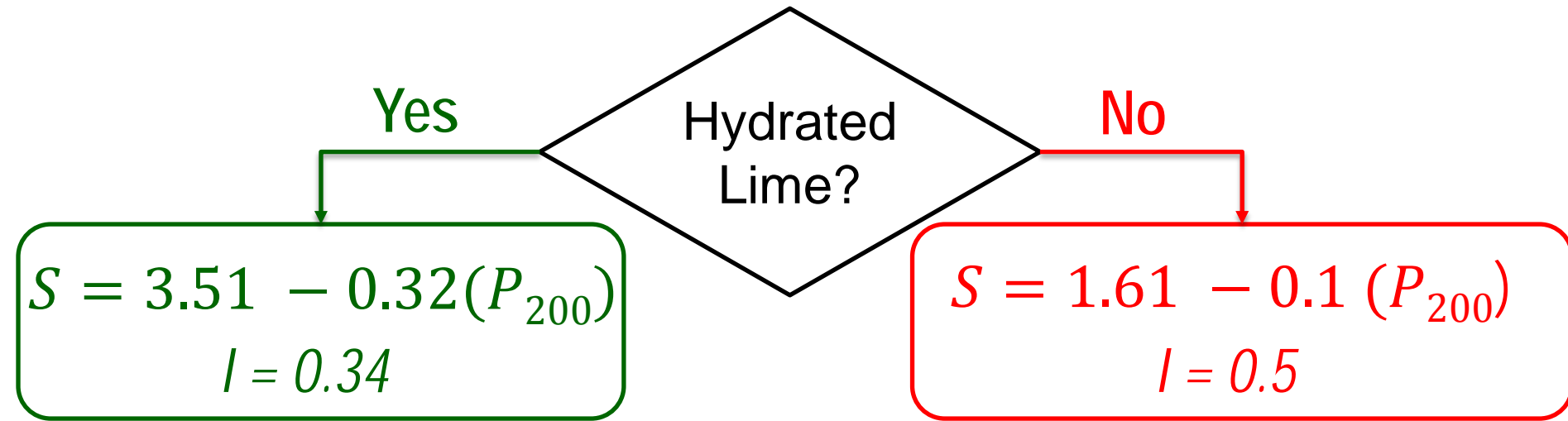
*Slow (Constant) Rate*

$$\log G^* = d(t) + e$$

*Loose Mix :  $a_l, b_l, c_l, d_l, e_l$*

*USAT :  $a_u, b_u, c_u, d_u, e_u$*

# Prediction Parameters



$$\log G^*_{fast\ rate} = a(t)^2 + b(t) + c \quad \log G^*_{slow\ rate} = d(t) + e$$

*Loose Mix Parameters*

$$a_l, b_l, c_l, d_l, e_l$$

$$a_l = aU \quad b_l = bU - F \quad c_l = cU - I$$

$$d_l = dU / S \quad e_l = e_U - I$$

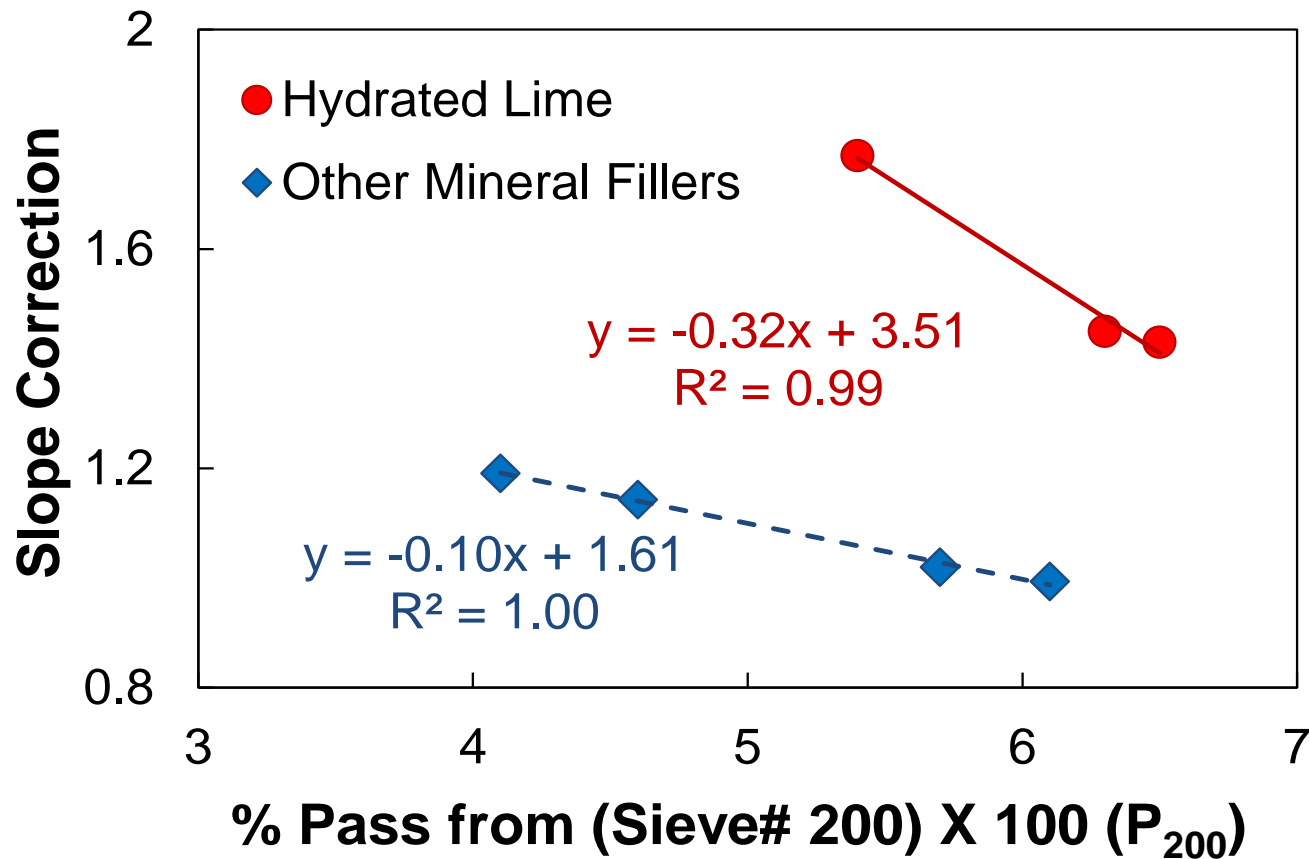
*USAT Parameters*

$$a_u, b_u, c_u, d_u, e_u$$

$$F = \frac{[(c_l + b_u \times 10 + a_l \times (10)^2) - (d_l \times 10 + e_l)]}{10}$$



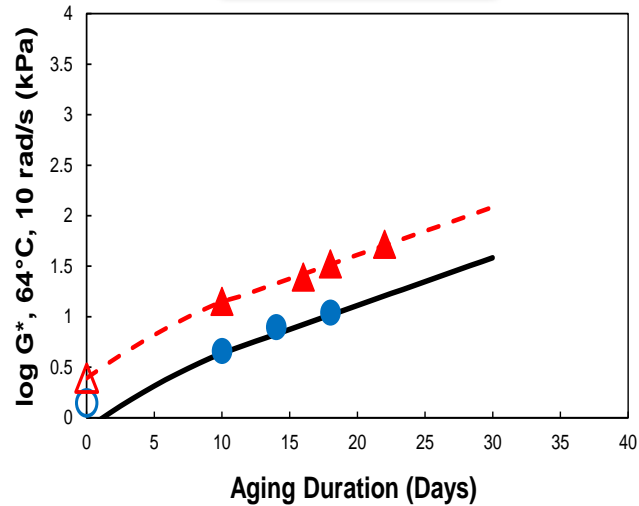
# Slope Correction (S)



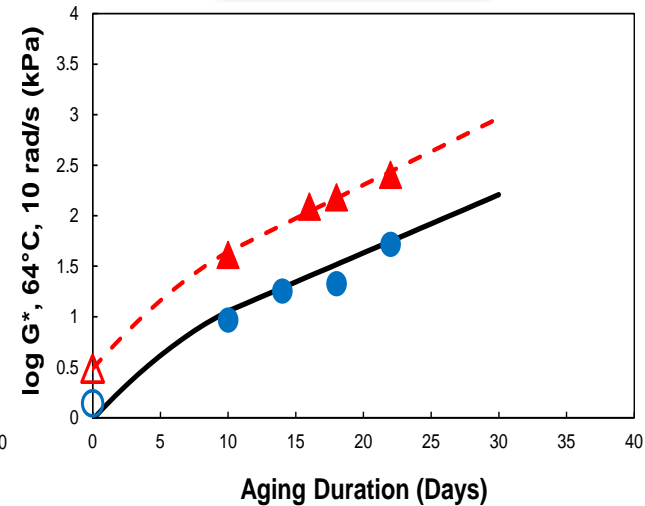
# Predicted Loose Mix Aging Rates No Lime



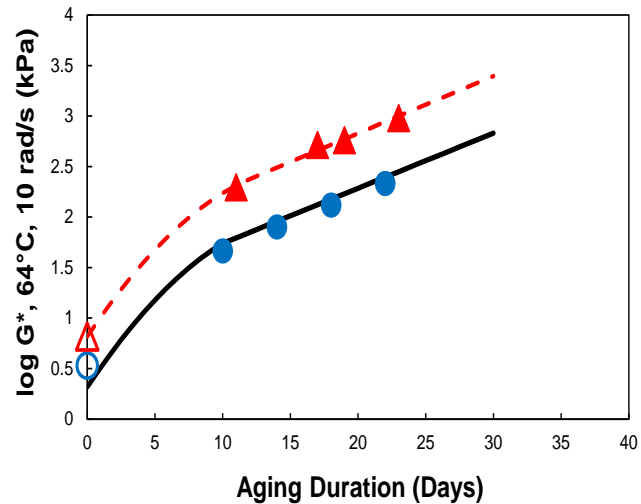
LTPP-NM



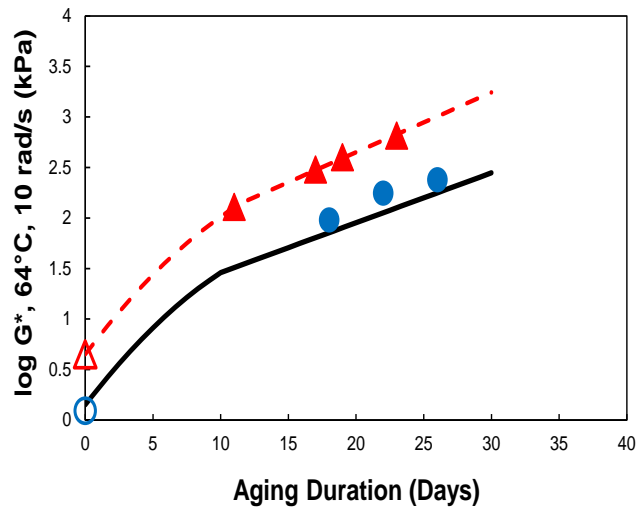
LTPP-SD



LTPP-TX



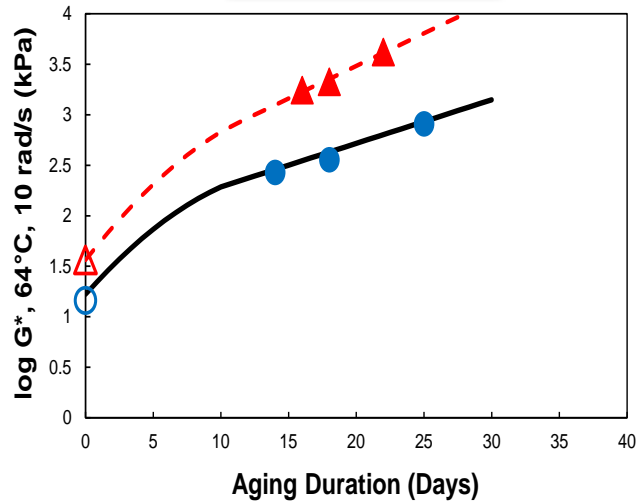
LTPP-WI



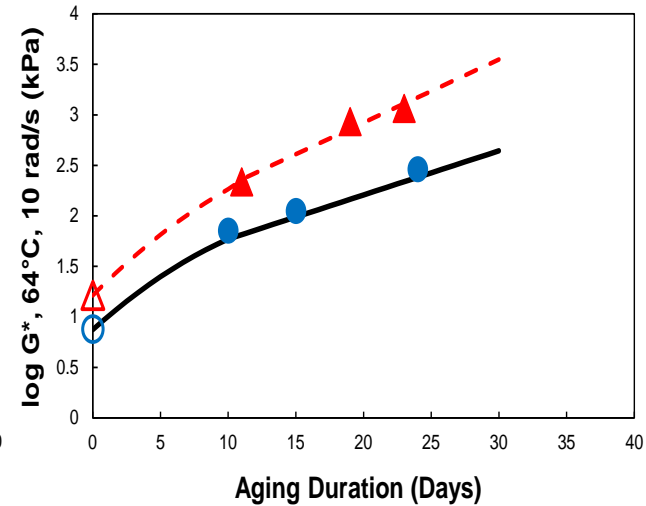
# Predicted Loose Mix Aging Rates With Lime



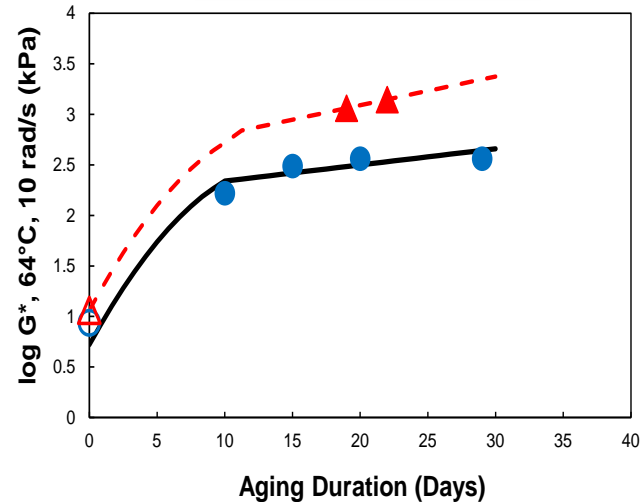
*ALF-Control*



*WesTrack-Coarse*



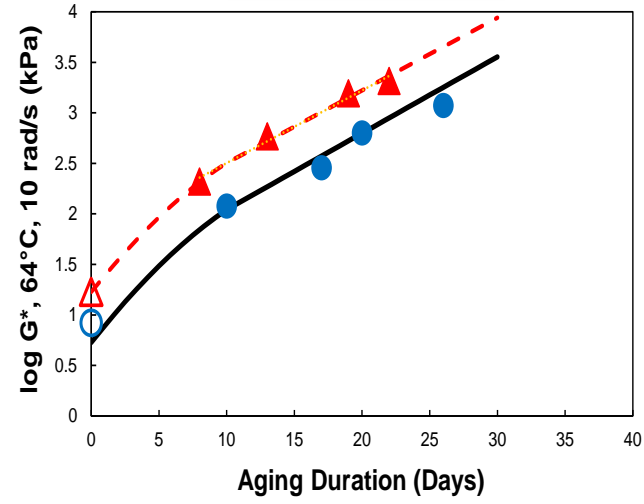
*WesTrack-Fine*



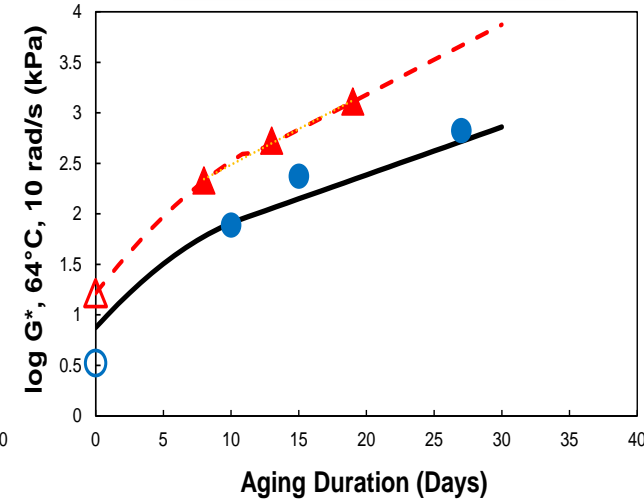
# Model Validation

- Loose Mix Aging
- ▲ Binder USAT Aging
- Prediction

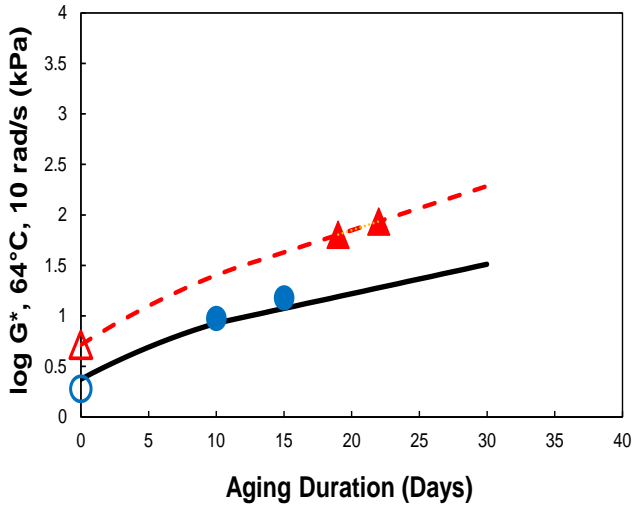
NCS9.5B



SHRP-AAD-1



SHRP-AAG-1



# Summary

- ❑ Oven aging of loose mixture at 95°C recommended
- ❑ G\*-based kinetics model developed and verified
- ❑ Aging duration maps developed using the climate data
- ❑ Draft AASHTO specification developed for long-term aging of asphalt mixtures for performance testing
- ❑ Preliminary pavement aging model established based on mixture-specific kinetics parameters and pavement depth dependent calibration factors
- ❑ Developed and verified the methodology to predict loose mixture aging from USAT aging

Questions?